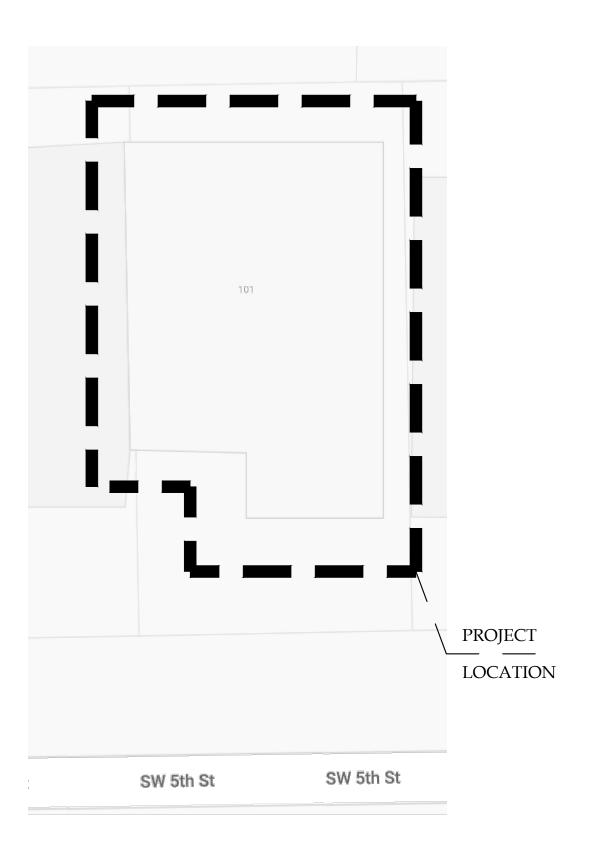
FIRE DAMAGE REPAIR

ADDRESS: 101 SW 5TH STREET POMPANO BEACH, FL 33060

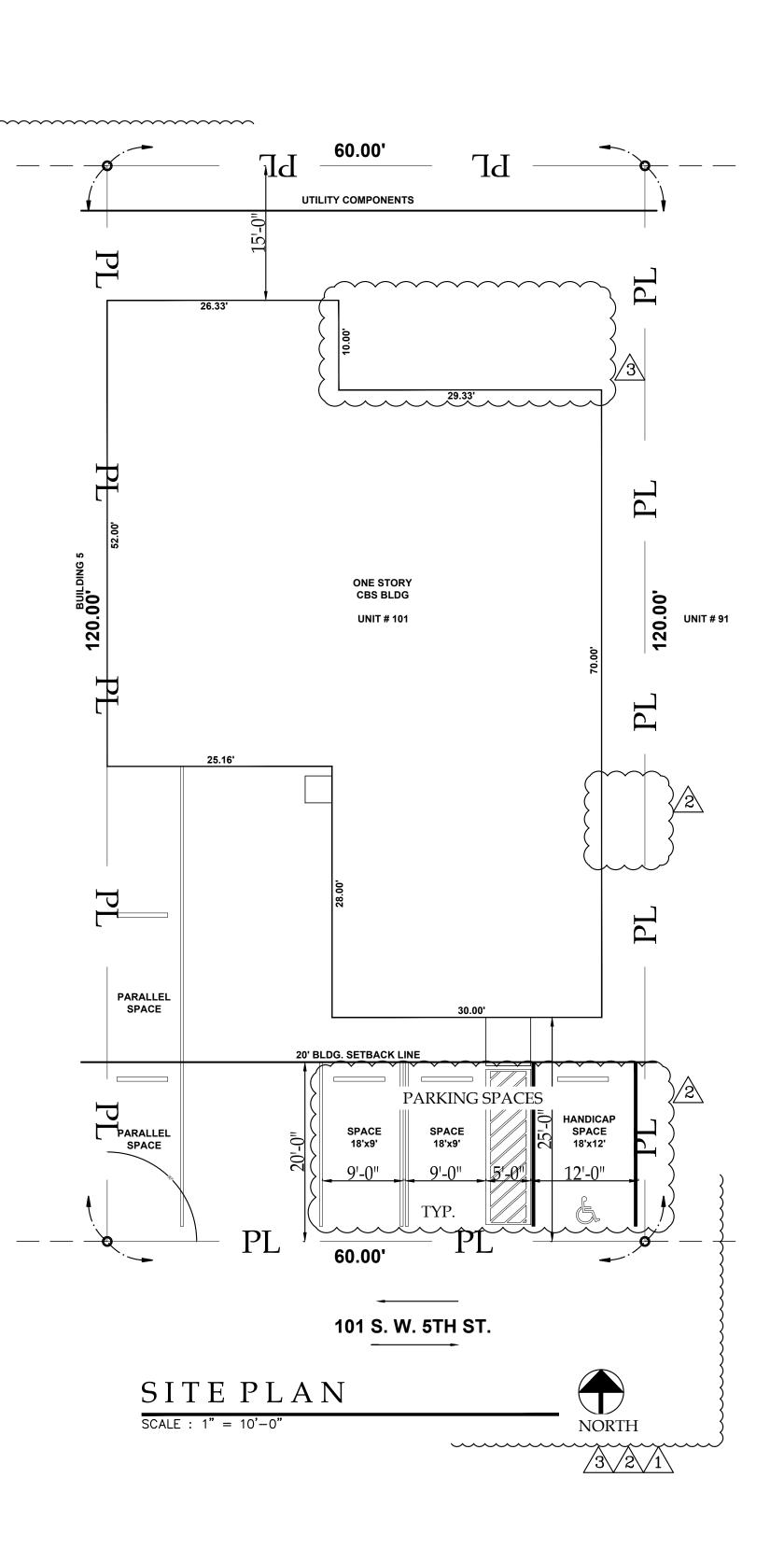
LEGAL DESCRIPTION: LOT 11 and the EAST 10 FEET OF LOT 12 in BLOCK 2 of POMPANO BEACH INDUSTRIAL CENTER, ACCORDING to the PLAT THEREOF, RECORDED in PLAT BOOK 39, PAGE 24, of the PUBLIC RECORDS of BROWARD COUNTY, FLORIDA.

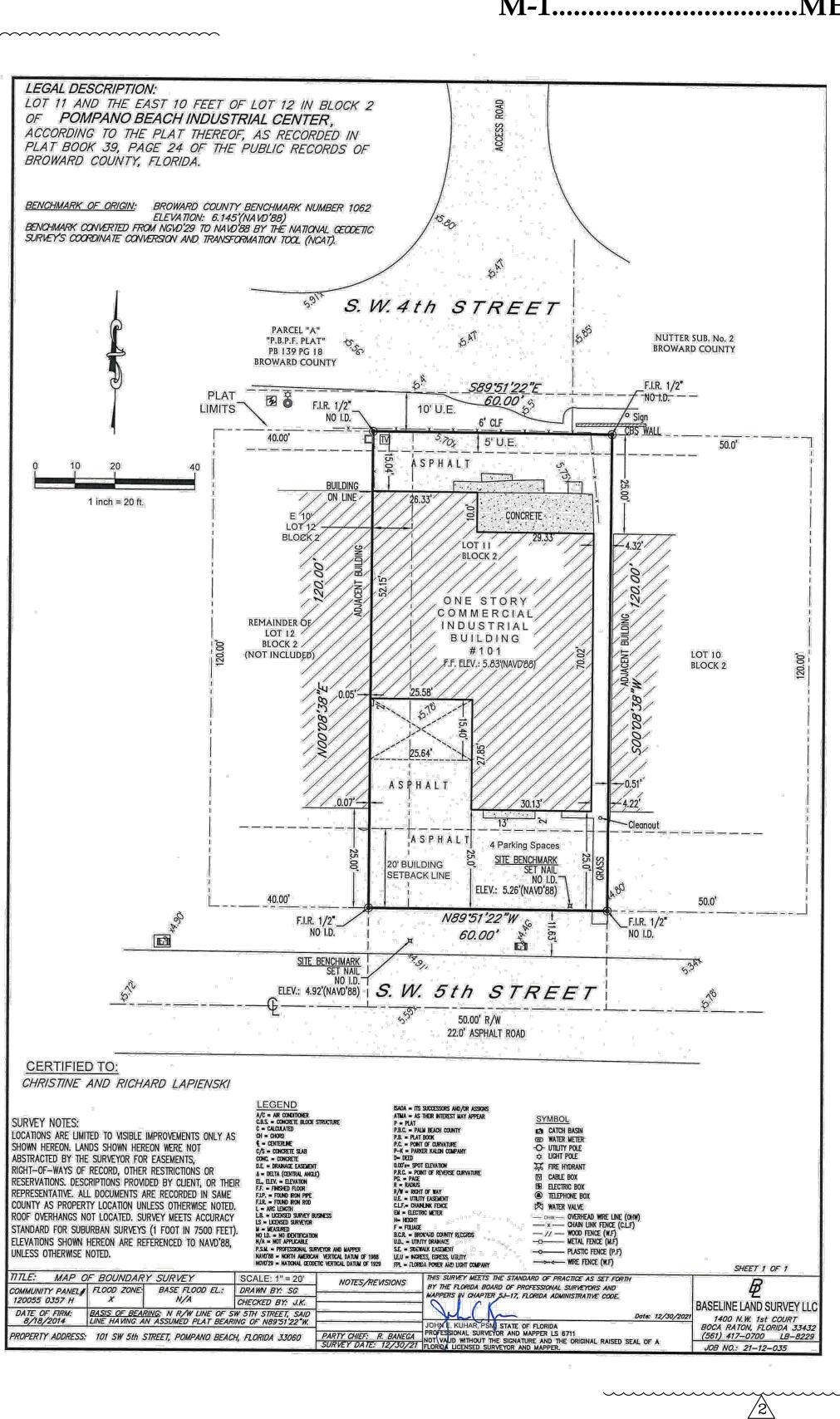




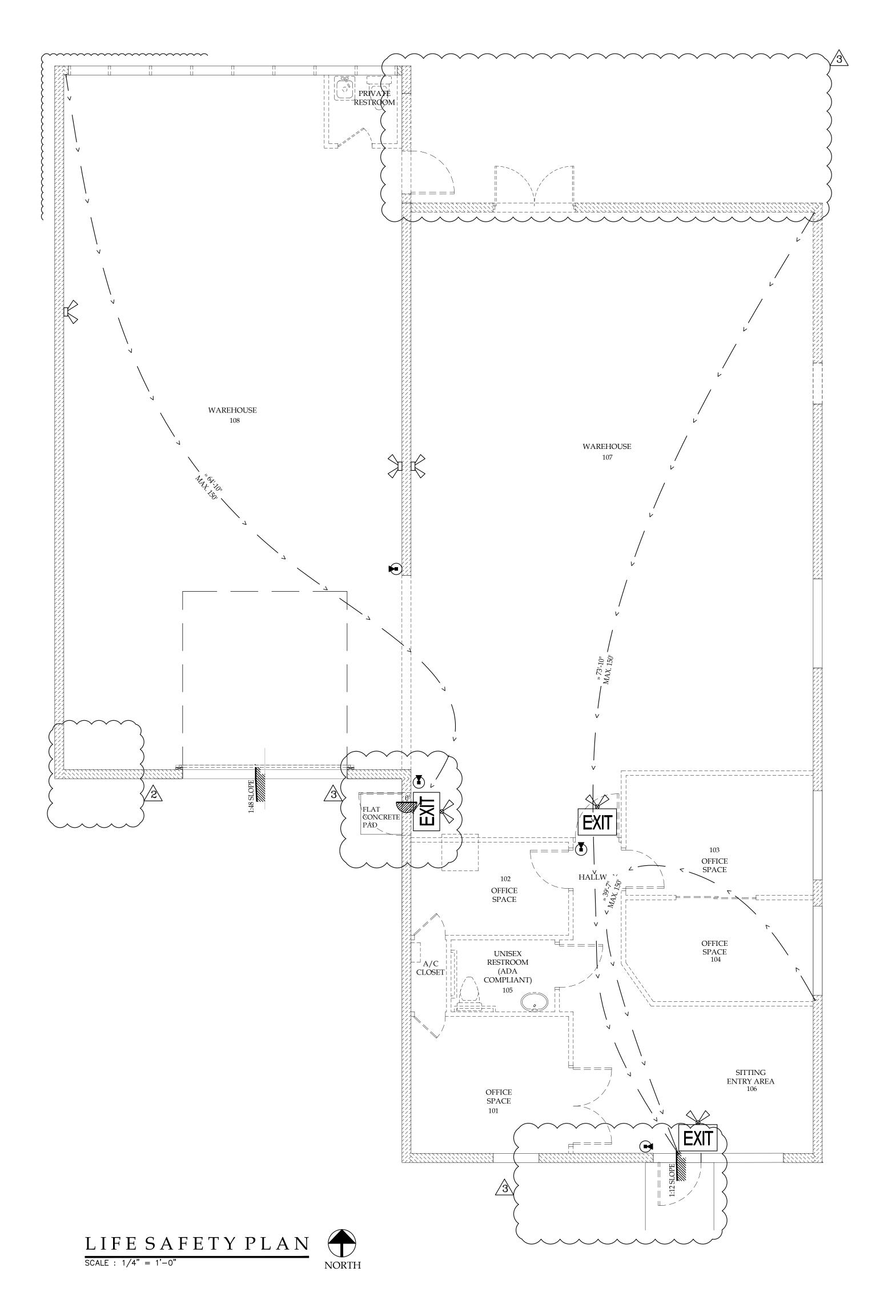


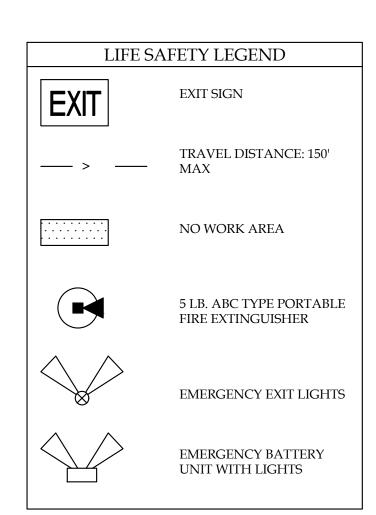
BRENNAN ENGINEERING, INC. 1600 S. DIXIE HWY. STE. 400B BOCA RATON, FL 33432 O: 561.392.3748





	INDEX
LS-1	LIFE SAFETY PLAN
S-0	DETAILS/GENERAL NOTES
S-1	DEMO/PROPOSED
S-2	
P-1	
E-1	ELECTRICAL
M-1	





NOTES: 1. ALL EXIT DOORS SHALL BE 3'-0" WIDE, 35" CLEAR WITH / .02 IN.

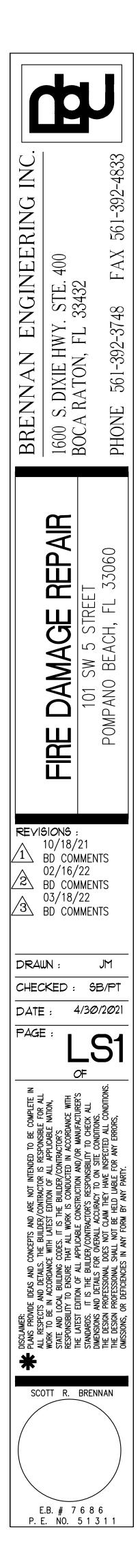
- PER PERSON AS PER TABLE 1004 OF THE F.B.C.
- FIRE EXTINGUISHER INSTALLED SO THAT MAXIMUM TRAVEL DISTANCE FROM ANY POINT IS NO MORE THAN 75 L.F.
 EXIT SINGS MUST COMPLY WITH NFPA 101 CHAPTER 7.

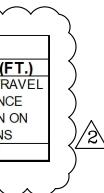
CODE DATA

USE AND OCCUPANCY CLASSIFICATION:	F-2
CONSTRUCTION TYPE:	V-B UNSPRINKLERED
BUILDING HEIGHT:	1 - STORY <15'
REQUIRED FIRE RESISTANCE OF RATING (TABLE 601 FB	C 7TH ED. (2020)):
EXTERIOR BEARING WALLS:	1 HR.
1. BEARING AND NON BEARING INTERIOR WALLS:	0 HR.
2. PARTY WALL SEPARATION:	1HR

NUMBER AN	D ARRANGM	ENT OF I	EXITS	
USE & OCCUPANCY	MINIMUMNU	MBER OF	TRAVEL DI	STANCE (FT.)
			ALLOWABLE	ACTUAL TRAVEL
BUSINESS, WAREHOUSE	REQ'D	EXITS	TRAVEL	DISTANCE
OCCUPANT LOAD FACTOR: 150, 500	REQD	EAIIS	DISTANCE	SHOWN ON
			(TABLE 1017)	PLANS
13 (SEE TABLE 1)	2	2	200'	94'

	Т	ABLE 1		
ROOM#	ROOM	AREA (S.F.)	#/NET PER PERSON	OCC LOAD
101	OFFICE	138	S.F./ 150 GROSS	1
102	OFFICE	93	S.F./ 150 GROSS	1
103	OFFICE	136	S.F./ 150 GROSS	1
104	OFFICE	111	S.F./ 150 GROSS	1
105	RESTROOM	40	S.F./ 100 GROSS	1
106	SITTING	109	S.F./ 100 GROSS	1
107	WAREHOUSE	1333	S.F./ 500 GROSS	3
108	WAREHOUSE	1334	S.F./ 500 GROSS	3
		TOTAL BUIL OCC LOAD		12





GENERAL NOTES:

THESE PLANS AND THE ASSOCIATED CALCULATIONS HAVE BEEN PREPARED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 7TH ED. (2020).

THE BUILDER SHALL REVIEW DRAWINGS IN THEIR ENTIRETY BEFORE STARTING WORK. THE BUILDER SHALL ACCEPT FULL RESPONSIBILITY FOR ANY ERRORS OR OMISSIONS NOT REPORTED IMMEDIATELY IN WRITING TO THIS ENGINEER. BY SUBMITTING THESE PLANS FOR PERMIT, THE BUILDER ACCEPTS FULL RESPONSIBILITY FOR THE ACCURACY OF THE INFORMATION DEPICTED WITHIN AND IT IS ASSUMED THAT THE BUILDER HAS READ AND UNDERSTANDS THE NOTES BELOW.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS.

CONSULT ARCHITECTURAL DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL IS SHOWN.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD.

DO NOT SCALE THE DRAWINGS. FOLLOW WRITTEN DIMENSIONS ONLY. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS WORK INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING TEMPORARY BRACING, GUYS OR TIEDOWNS.

THE CONTRACTOR SHALL SUPPLEMENT THE MINIMUM REQUIRED FOUNDATION AND SITE PREPARATION REQUIREMENTS AND SLAB-ON-GRADE THICKNESS TO HANDLE CONSTRUCTION LOADS.

THE STRUCTURAL FRAMING SHOWN IS SCHEMATIC IN NATURE. HOWEVER THE SUPPORTING STRUCTURE HAS BEEN DESIGNED UNDER THE ASSUMPTION THE FRAMING SCHEME SHOWN WILL CLOSELY PARALLEL FINAL FLOOR DESIGNERS LAYOUT. PROVIDE SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL. REVIEW OF SHOP DRAWINGS IS REQUIRED AS THE ENGINEER CHECKS BEAM, WALL, COLUMN AND FOOTING CAPACITIES AGAINST KNOWN LOADING. FAILURE TO PROVIDE SHOP DRAWINGS MAY RESULT IN ADDITIONAL ENGINEERING COSTS DURING CONSTRUCTION.

SHOP DRAWING REVIEW

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY

LENGTH, ELEVATIONS, DIMENSIONS, ETC. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS

SHOP DRAWINGS IN THE FORM OF REPRODUCIBLE SEPIAS OF STRUCTURAL DRAWINGS (CONTRACT DOCUMENTS) ARE PROHIBITED WITHOUT THE EXPRESS WRITTEN PERMISSION FROM THE ENGINEER.

IN ALL INSTANCES, THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS CHECKED, UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER.

GRAVITY LOADING SUPERIMPOSED: ROOF LIVE LOAD = 30 PSF ROOF DEAD LOAD = 25 PSF

CONCRETE:

CONCRETE DESIGN HAS BEEN PERFORMED IN ACCORDANCE WITH ACI 318-14, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

NORMAL WEIGHT CONCRETE (145 PCF) 28-DAY COMPRESSIVE STRENGTH:

	MIN	MA
MASSIVE SECTIONS, PAVEMENTS, AND SLABS	1-1/2	4
HEAVY SLABS, BEAMS, WALLS	3	5
THIN WALLS, COLUMNS,	$3\frac{1}{2}$	6

SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.

MINIMUM CONCRETE CLEAR COVER SHALL BE:

BEAMS AND COLUMNS - 1-1/2"

TIE COLUMNS - 3/4"

EXPOSED UNPROTECTED CONCRETE - 1-1/2" FORMED CONCRETE BELOW GRADE - 2"

UNFORMED CONCRETE BELOW GRADE - 3"

PLACING DRAWINGS AND BAR LISTS SHALL CONFORM TO A.C.I.'S "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES." (A.C.I. 315)

DETAILS OF CONCRETE REINFORCEMENT SHALL BE IN ACCORDANCE WITH "THE MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION" AS PUBLISHED BY THE CONCRETE REINFORCING STEEL INSTITUTE UNLESS OTHERWISE INDICATED.

CONCRETE CONSTRUCTION TECHNIQUES SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301).

ADEQUATE VERTICAL AND HORIZONTAL SHORING SHALL BE PROVIDED TO SAFELY SUPPORT ALL CONSTRUCTION LOADS.

CONCRETE BEAM SIZES MAY BE INCREASED AS REQUIRED FOR ARCHITECTURAL DETAILS OR TO FIT BLOCK COURSING.

CONTRACTOR SHALL SUBMIT PROPOSED MIX DESIGNS, WITH HISTORICAL STRENGTH DATA FOR EACH SEPARATE MIX PRIOR TO CONCRETE PLACEMENT.

CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ACI 301 AND ASTM C-94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME-STAMPED WHEN CONCRETE IS BATCHED. THE MAXIMUM TIME ALLOWED FROM WHEN WATER IS ADDED TO THE MIX UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED 90 MINUTES. IF FOR ANY REASON THERE IS A

DELAY SUCH THAT A BATCH IS HELD FOR LONGER THAN 90 MINUTES, THE CONCRETE SHALL NOT BE PLACED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LABORATORY TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE.

ALL CONCRETE SHALL BE CURED USING A CURING COMPOUND MEETING ASTM STANDARD C-309, TYPE 1. CURING COMPOUNDS SHALL HAVE A FUGITIVE DYE. THE CURING COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE VISIBLE WATER HAS LEFT THE UNFINISHED CONCRETE.

ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY.

CALCIUM CHLORIDES SHALL NOT BE UTILIZED IN THE WORK.

OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

FORMWORK

FORMWORK. SHORING, AND BRACING FOR ALL CONCRETE BEAMS, SLABS, COLUMNS, WALLS, AND FOOTINGS SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH ACI 347, "RECOMMENDED PRACTICE FOR CONCRETE FORMWORK".

REINFORCING STEEL

REBAR SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE, AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF THE ACI STANDARDS AND SPECIFICATIONS.

CONTRACTOR SHALL SUBMIT REBAR SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.

UNSCHEDULED FIELD LAPS ARE SUBJECT TO ENGINEER'S REVIEW. TENSION LAP SPLICE SCHEDULE

MASONR BAR No. SPICE LEN

42"

SAME SIZE AS LONGITUDINAL BEAM BARS. SEE DETAILS FOR ADDITIONAL INFORMATION.

REINFORCING STEEL IN FOOTINGS SHALL BE ASSEMBLED AS MATS WITH BARS EQUALLY SPACED AND WIRED TOGETHER AT EACH INTERSECTION BEFORE CONCRETE IS PLACED.

ABOVE.

WELDED WIRE MESH:

WITH THE ACI TYPICAL DETAILS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES. MASONRY:

STRUCTURAL DESIGN IS IN ACCORDANCE WITH A.C.I.530-13/ASCE 5-13/TMS 402-16, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND THE COMMENTARY ON BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.

CONSTRUCTION SHALL BE IN ACCORDANCE WITH A.C.I.530-13/ASCE 6-13/TMS 602-16, SPECIFICATIONS FOR MASONRY STRUCTURES AND THE COMMENTARY ON SPECIFICATIONS FOR MASONRY STRUCTURES.

ALL BLOCK WALLS SHALL BE TWO-CELL HOLLOW CONCRETE MASONRY REGULAR SIZE BLOCK MANUFACTURED IN CONFORMANCE WITH ASTM C-90, GRADE N, F'm=1500 PSI.

BLOCK SHALL BE PLACED IN RUNNING BOND UNLESS OTHERWISE NOTED. LAY UP MASONRY WALLS TO BOTTOM OF TIE BEAMS BEFORE PLACING CONCRETE FOR IN WALL COLUMNS.

GROUT USED TO FILL MASONRY CELLS SHALL COMPLY WITH ASTM C-476 AND SHALL PROVIDE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS. THE GROUT MIX SHALL HAVE A MAXIMUM 3/8" COURSE AGGREGATE AND SHALL BE PLACED WITH A SLUMP OF 8" TO 10". USE MECHANICAL VIBRATION TO CONSOLIDATE GROUT.

TYPE S MORTAR SHALL BE USED EXCLUSIVELY ON THIS PROJECT. MORTAR SHALL BE PROPORTIONED AND MIXED AS OUTLINED UNDER ASTM C-270. HORIZONTAL AND VERTICAL MORTAR JOINTS SHALL BE 3/8" THICK UNLESS OTHERWISE NOTED. REMOVE MORTAR PROTRUSIONS THAT EXTEND INTO THE CELLS TO BE FILLED. ALLOW A MINIMUM OF 24 HOURS FOR MORTAR TO CURE PRIOR TO GROUTING CELLS.

HORIZONTAL MORTAR JOINTS SHALL BE REINFORCED WITH STANDARD 9 GAGE LADDER-TYPE DUR-O-WAL (ASTM CLASS B-2, HOT DIPPED GALVANIZED) AT ALTERNATE COURSES (16" ON CENTER), UNLESS OTHERWISE NOTED. JOINT REINFORCEMENT SHALL BE CONTINUOUS AND SHALL LAP A MINIMUM OF 8".

TENSION AND COMPRESSION LAPS = .002 X BAR DIAMETER X fs (NOTE THAT fs=24 KSI MAXIMUM (.4 X Fy)) (THIS EQUATES TO 48 X BAR DIAMETER)

MASONRY CONSTRUCTION SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A "CERTIFIED STRUCTURAL MASONRY CONTRACTOR". THE SUPERVISOR OF THE MASONRY PORTION OF THE PROJECT SHALL BE A "CERTIFIED STRUCTURAL MASONRY CONTRACTOR" OR A "CERTIFIED STRUCTURAL MASON" AS RECOGNIZED BY THE FLORIDA CONCRETE AND PRODUCTS ASSOCIATION (FC&PA). THE SENIOR MASONRY SUPERVISOR WILL BE RESPONSIBLE TO ASSURE THAT THE WORK IS ACCOMPLISHED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE MASONRY CONTRACTOR SHALL SUBMIT CREDENTIALS FOR THE FC&PA TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO BIDDING.

REINFORCING BAR SHALL BE IN ALL WALL INTERSECTIONS AND AT EACH SIDE OF ALL OPENINGS.

REINFORCING BARS SHALL BE AT ALL GIRDER TRUSSES WITH GRAVITY LOADS GREATER THAN 3500 LBS

TAG	TYPE	MANUFA
M1	TUBE	CGI Windows and

TAG	TYPE	MANUFACTURI
W1	FIXED WINDOW	CGI Windows and Doors, Inc
W2	FIXED WINDOW	CGI Windows and Doors, Inc
W3	STOREFRONTS	ALDORA

				DOC	DR SC	HEDU	LE					
TAG	TYPE	MANUFACTURER	MODEL	OPENING SIZE (W X H)	MAX PRODU			PRESSURE	IMPACT	NOA	EXPIRATION	NOTES
				, ,	+PSF	-PSF	+PSF	-PSF		#	DATE	
D1	OVERHEAD DOOR	AMARR GARAGE DOORS	2402 STEEL	192" x 156"	45	52	34	43	YES	IBC-2416-180-26-I	02/13/2024	UP TO 16'-2" WIDI
D2	STOREFRONT DOOR	ALDORA	ALUMINUM	36" x 84"	100	120	36	39	YES	20-0828.08	03/12/2022	-
D3	INTERIOR DOOR	SWING (SELF CLOSING)	WOOD	36" x 80"	-	-	-	-	NO	-	-	FIRE RATED
D4	INTERIOR DOOR	PAIR SWING	WOOD	(2) 30" x 80"	-	-	-	-	NO	-	-	-
D5	INTERIOR DOOR	SWING	WOOD	24" x 80"	-	-	-	-	NO	-	-	-
D6	INTERIOR DOOR	SWING	WOOD	36" x 80"	-	-	-	-	NO	-	-	-
D7	EXTERIOR DOOR	SCHLAGE LOCK	METAL	36" x 80"	120	120	38	41	YES	21-1228.05	02/24/2027	FIRE RATED
D8	EXTERIOR DOOR	NEXT DOOR COMPANY	METAL	36" x 84"	75	75	36	39	YES	21-0504.09	05/13/2024	FIRE RATED
D9												

PILING 5000 PSI WITH .40 WATER - CEMENT RATIO SHALLOW FOUNDATIONS 2500 PSI. PILE SUPPORTED FOUNDATIONS 5000 PSI WITH .40 WATER TO CEMENT RATIO COLUMNS, BEAMS, AND ELEVATED SLABS: 3000 PSI CONCRETE SLUMP SHALL BE AS FOLLOWS: SLUMP IN INCHES

SLAB - 3/4"

HORIZONTAL AND VERTICAL BARS SHALL LAP 6 x BAR NO., SEE LAP SPLICE SCHEDULE.

-				
	Р	LAIN REINF	ORCEMENT	
RY	OTHER TH	IAN TOP BAF	RS TOP BA	ARS
NGTH	(3000 PSI)	(4000 PSI)	(3000 PSI)	(4000 PS
	16"	16"	21"	18"
	22"	19"	28"	24"
	27"	23"	35"	30"
	35"	31"	46"	40"

42"

PROVIDE 3' x 3' CORNER BARS LAPPED AND TIED TO EACH BEAM REBAR, TYPICAL AT ALL CORNERS. THESE CORNER BARS SHALL BE THE

54"

REINFORCEMENT SHALL BE CAREFULLY PLACED, RIGIDLY SUPPORTED AND WELL TIED WITH BAR SUPPORTS AND SPACERS.

63"

DOWEL COLUMN AND WALL REINFORCING TO FOOTING WITH SAME SIZE AND NUMBER OF DOWELS AS VERTICAL BARS ABOVE.

DOWELS SHALL BE HOOKED "L" AT BOTTOM AND SHALL BE LAPPED 48 BAR DIAMETER WITH THE COLUMN OR WALL REINFORCING

CONCRETE COLUMNS SHALL BE TIED COLUMNS UNLESS OTHERWISE INDICATED.

WELDED WIRE MESH SHALL BE ASTM A185, GRADE 65, FREE FROM OIL, SCALE, AND RUST, AND SHALL BE PLACED IN ACCORDANCE

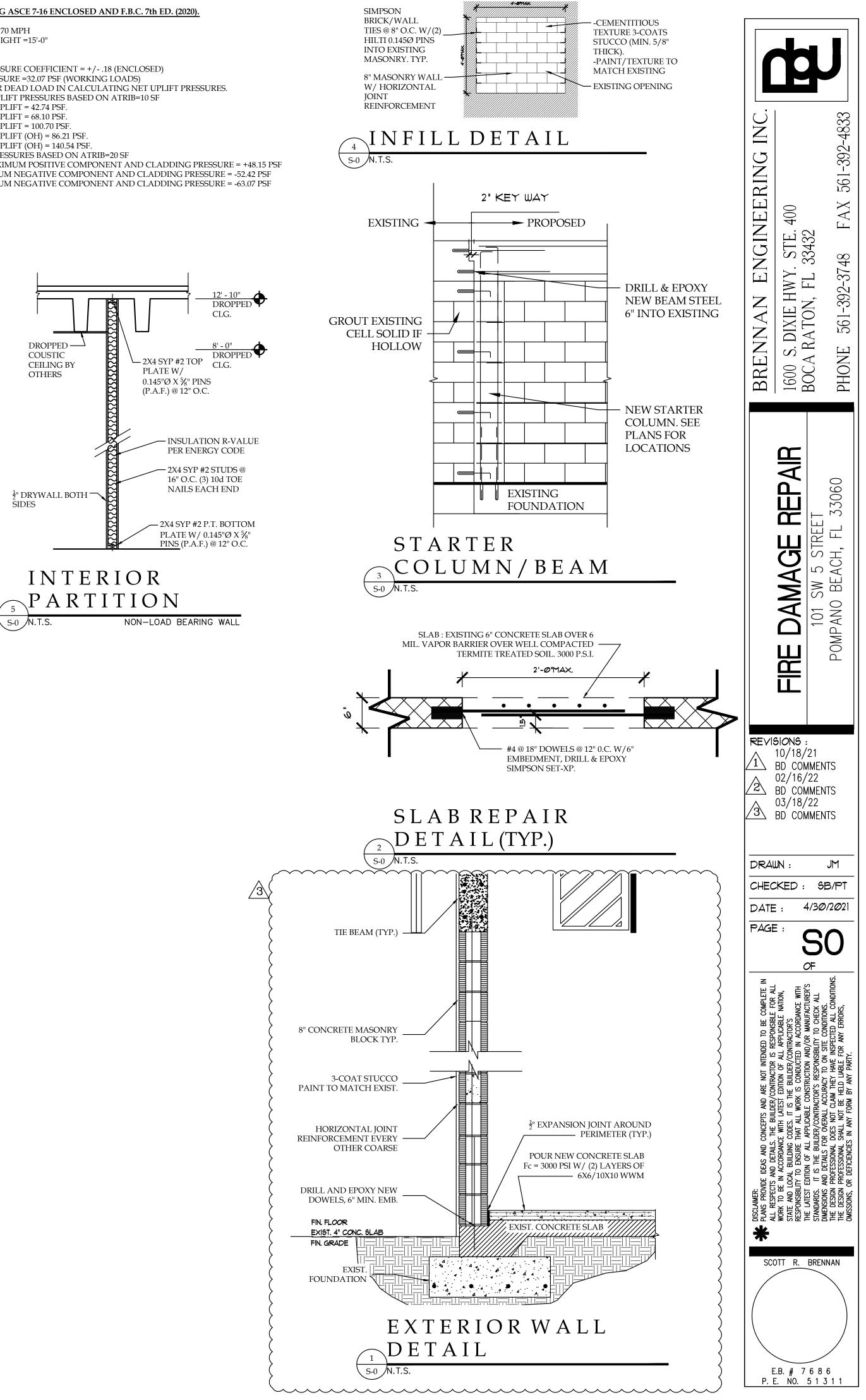
		MULLI	ON SO	CHED	ULE					
CTURER	MODEL	SIZE (WXH)	MAX PRODU	CT PRESSURE	DESIGN PRESSURE		IMPACT	NOA #	EXPIRATION	NOTES
		· · · · ·	+PSF	-PSF	+PSF	-PSF			DATE	
Doors, Inc	-	1" x 4"	130	130	38	41	YES	18-0129	03/28/2023	-

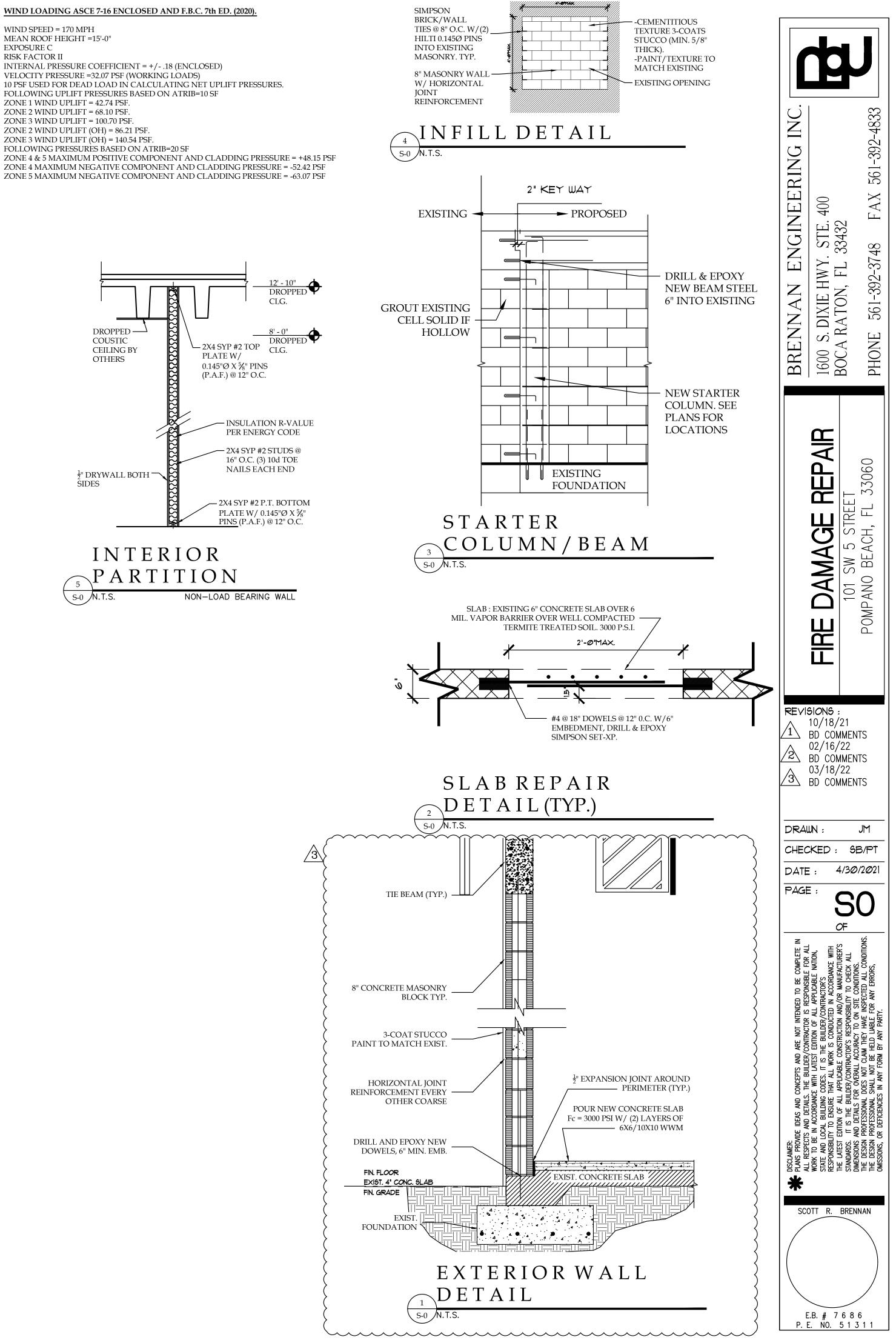
WINDOW SCHEDULE

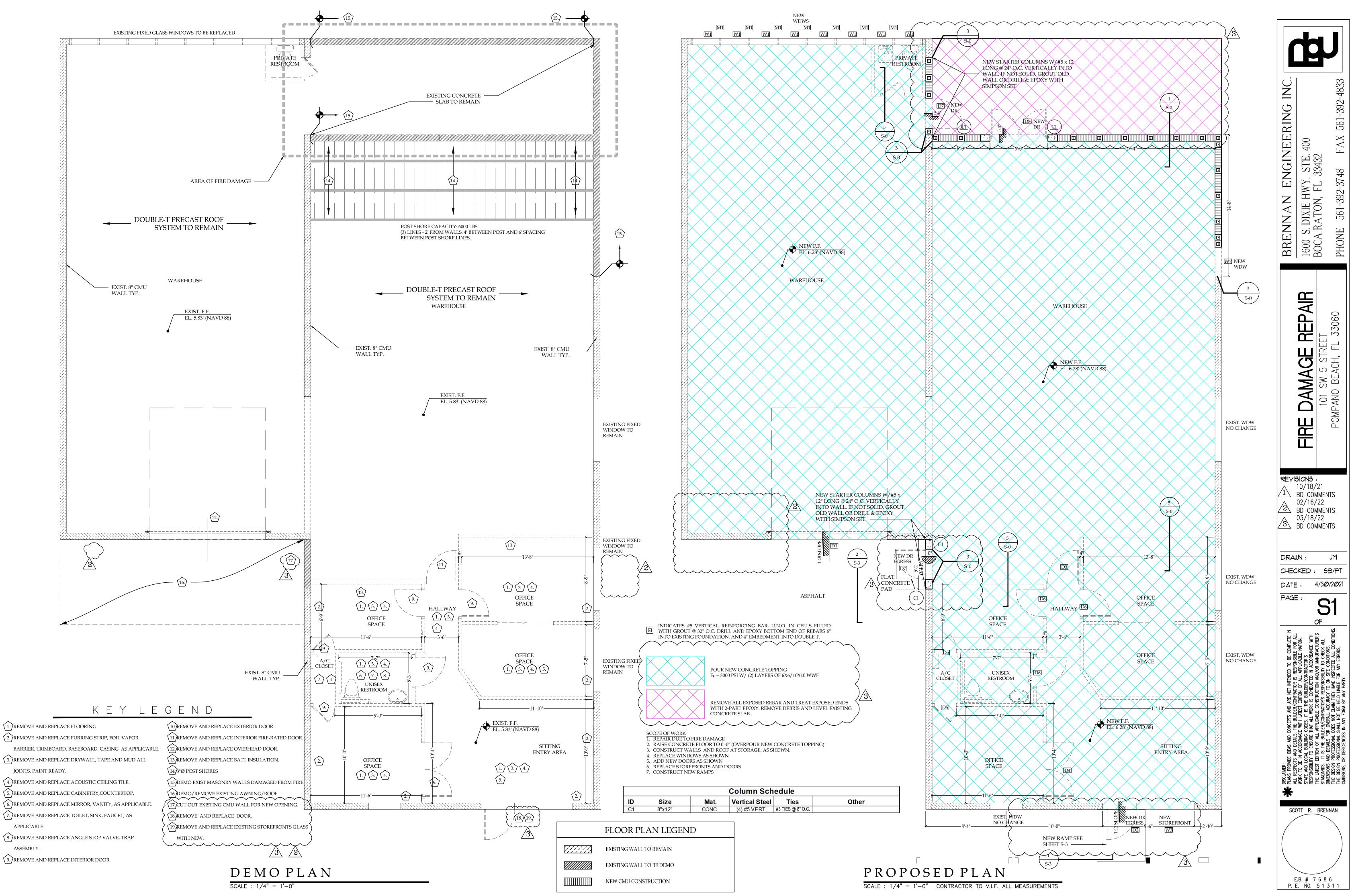
JRER	MODEL	OPENING SIZE (WXH)	MAX PRODU	CT PRESSURE	DESIGN I	PRESSURE	IMPACT	NOA	EXPIRATION	NOTES
		· · · · ·	+PSF	-PSF	+PSF	-PSF		#	DATE	
vs	PW-4120	36" x 36"	55	65.0	55	65	YES	FL26095-R1	02/19/2023	-
nc										
vs nc	PW-4120	36" x 36"	55	65.0	55	65	YES	FL26095-R1	02/19/2023	-
	ALUMINUM	36" x 36"	120	135	38	51	YES	20-0910.06	07/03/2024	-
	ALUMINUM	72" x 36"	120	135	38	51	YES	20-0910.06	07/03/2024	-
	ALUMINUM	72" x 52"	120	135	38	51	YES	20-0910.06	07/03/2024	-
	ALUMINUM	72" x 32"	120	135	38	51	YES	20-0910.06	07/03/2024	-

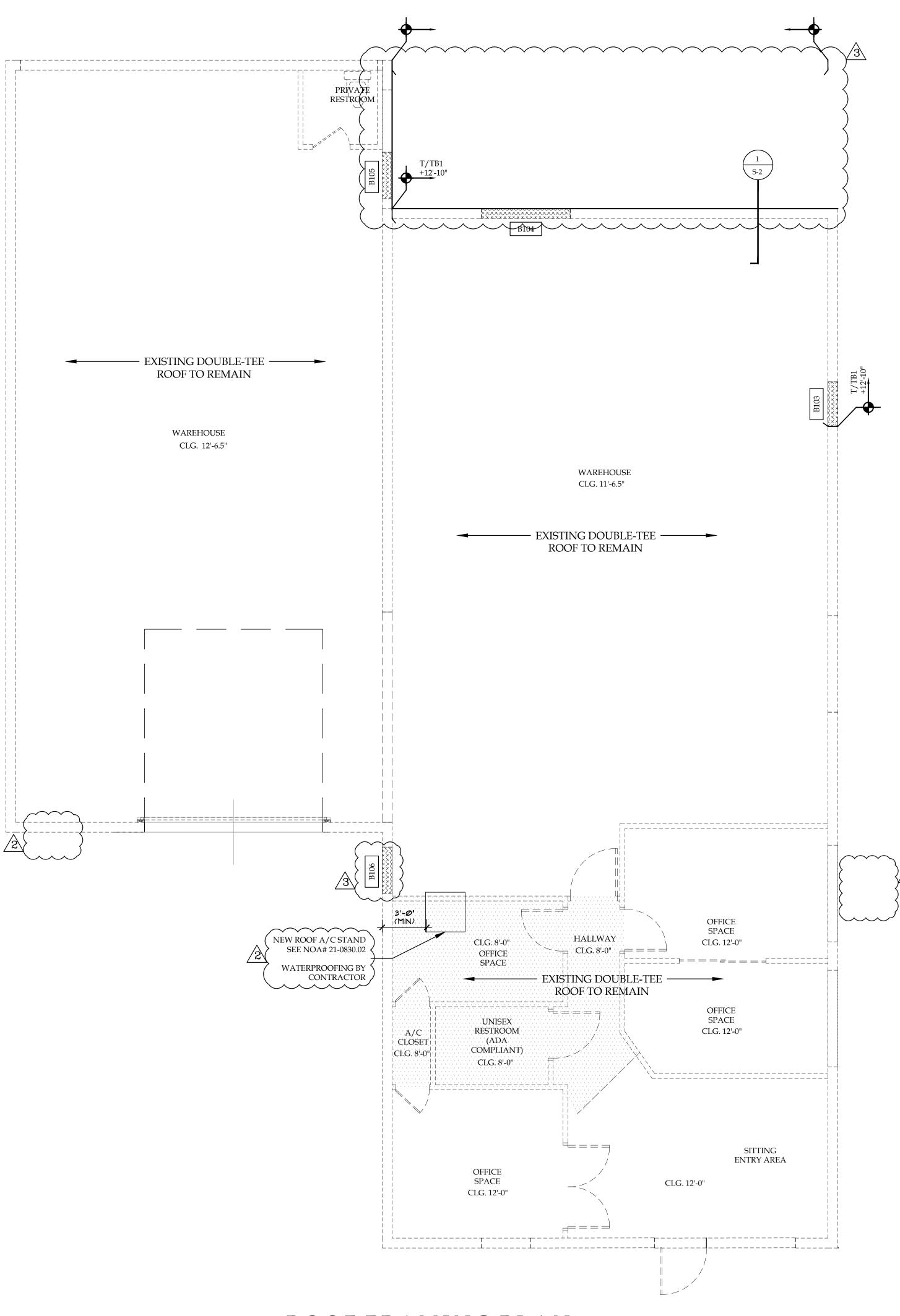
WIND LOADING ASCE 7-16 ENCLOSED AND F.B.C. 7th ED. (2020)

WIND SPEED = 170 MPH MEAN ROOF HEIGHT =15'-0" EXPOSURE C **RISK FACTOR II** INTERNAL PRESSURE COEFFICIENT = +/- .18 (ENCLOSED) VELOCITY PRESSURE =32.07 PSF (WORKING LOADS) 10 PSF USED FOR DEAD LOAD IN CALCULATING NET UPLIFT PRESSURES. FOLLOWING UPLIFT PRESSURES BASED ON ATRIB=10 SF ZONE 1 WIND UPLIFT = 42.74 PSF. ZONE 2 WIND UPLIFT = 68.10 PSF. ZONE 3 WIND UPLIFT = 100.70 PSF. ZONE 2 WIND UPLIFT (OH) = 86.21 PSF ZONE 3 WIND UPLIFT (OH) = 140.54 PSF. FOLLOWING PRESSURES BASED ON ATRIB=20 SF ZONE 4 & 5 MAXIMUM POSITIVE COMPONENT AND CLADDING PRESSURE = +48.15 PSF



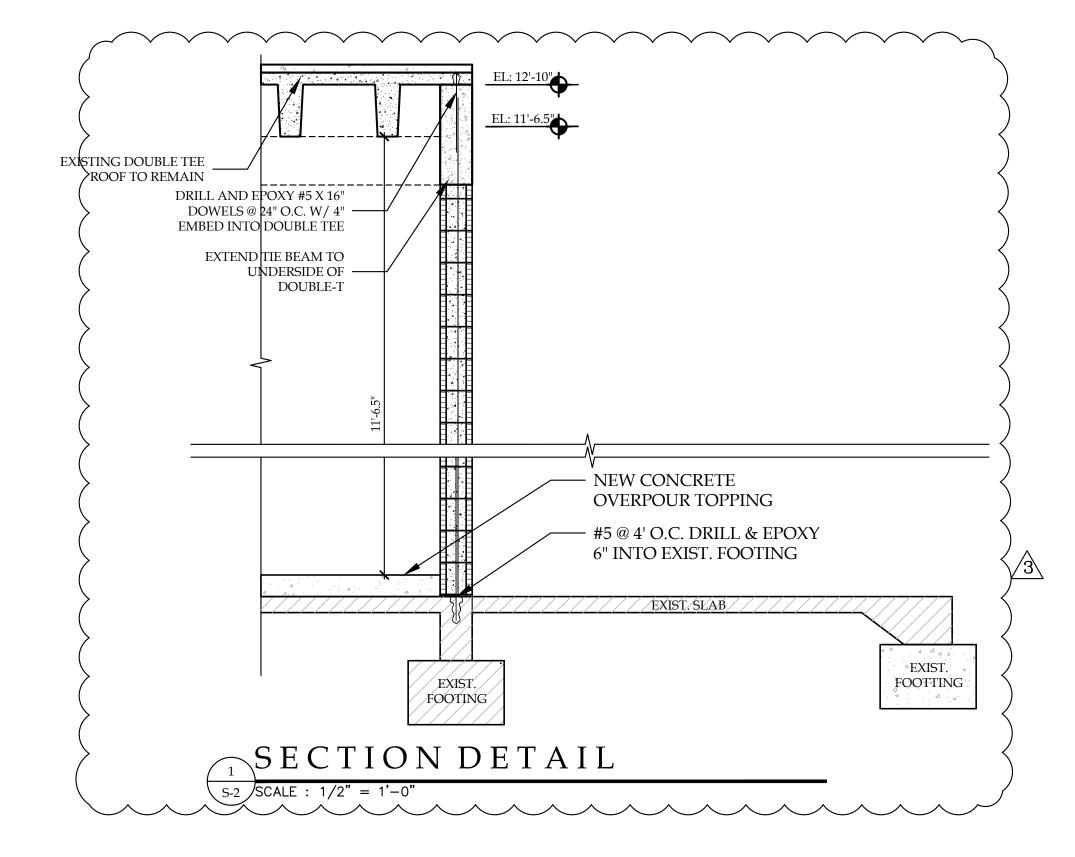


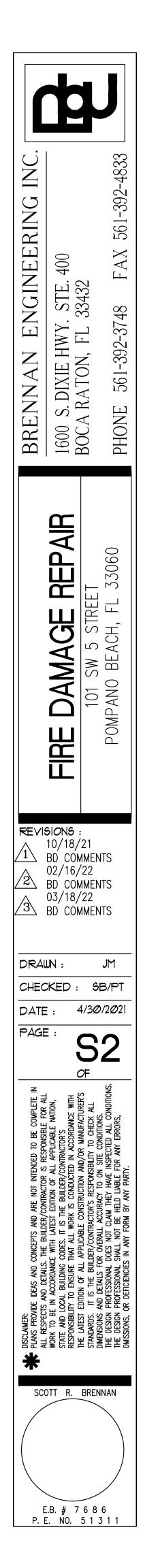


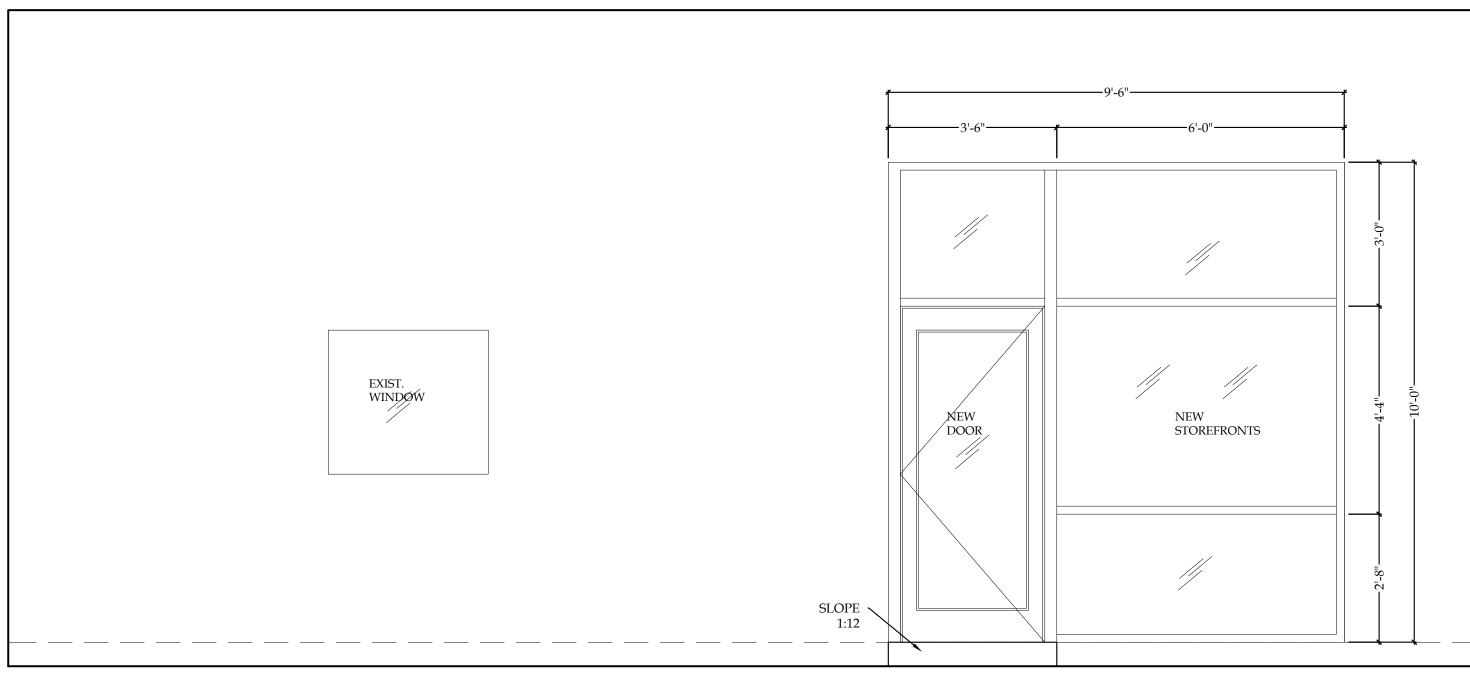


 $\frac{ROOF FRAMING PLAN}{SCALE : 1/4" = 1'-0"}$

	Beam Schedule												
ID	Width	Depth	TOP	MI	D	BOT	STIRRUPS	NOTES	Elevation				
TB1	8	12	(2) #5	-	-	(2) #5	#3 STIRRUPS @ 24" O.C.						
B101	8	24	(2) #5	2	#5	(2) #5	#3 STIRRUPS @ 24" O.C.						
B102	8	24	(2) #5	2	#5	(2) #5	#3 STIRRUPS @ 24" O.C.						
B103	8	24	(2) #5	2	#5	(2) #5	#3 STIRRUPS @ 12" O.C.						
B104	~ <mark>%</mark> ~~	24	(2) #5	2	<u>#5</u>	(2) #5	#3 STIRRUPS @ 12" O.C.						
B105	8	16	(2) #5	I	-	(2) #5	#3 STIRRUPS @ 8" O.C.						
·B106个	$\sim \frac{1}{8} \sim$		(2) ^{#5} ∕			(2) [*] #5	Î#3 ŜTIRRUPS @ 8" Ô.C.						

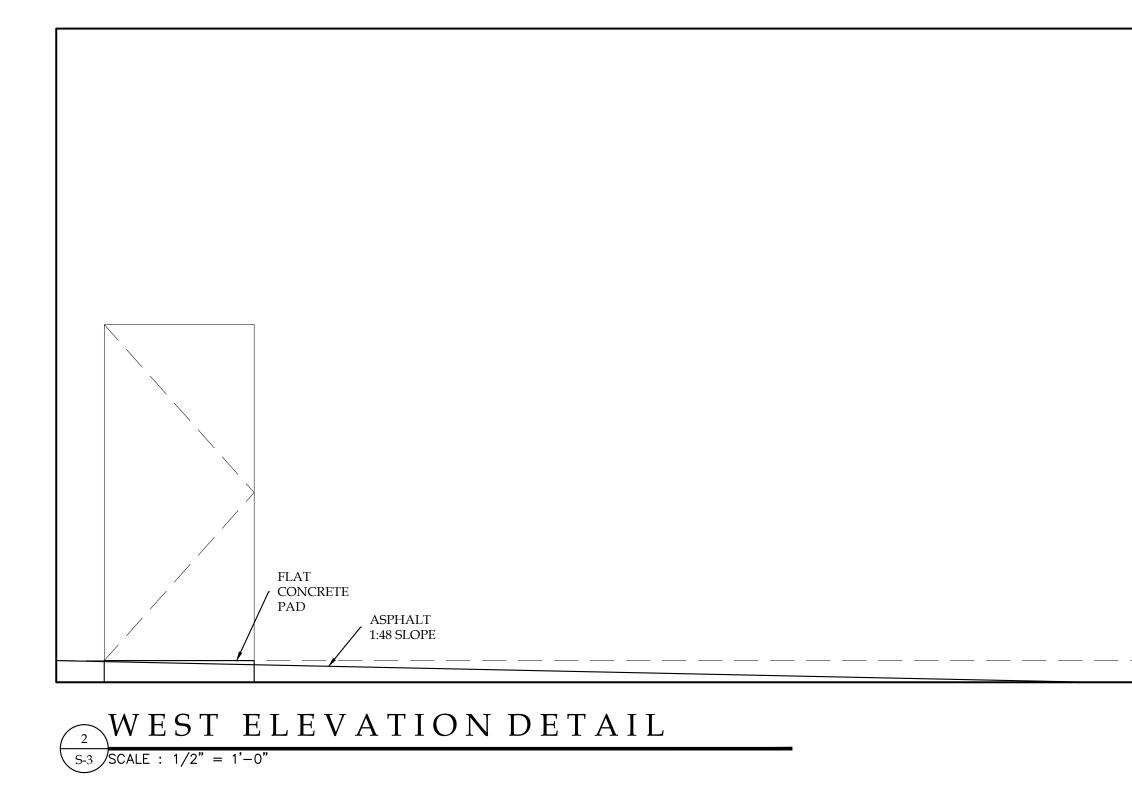


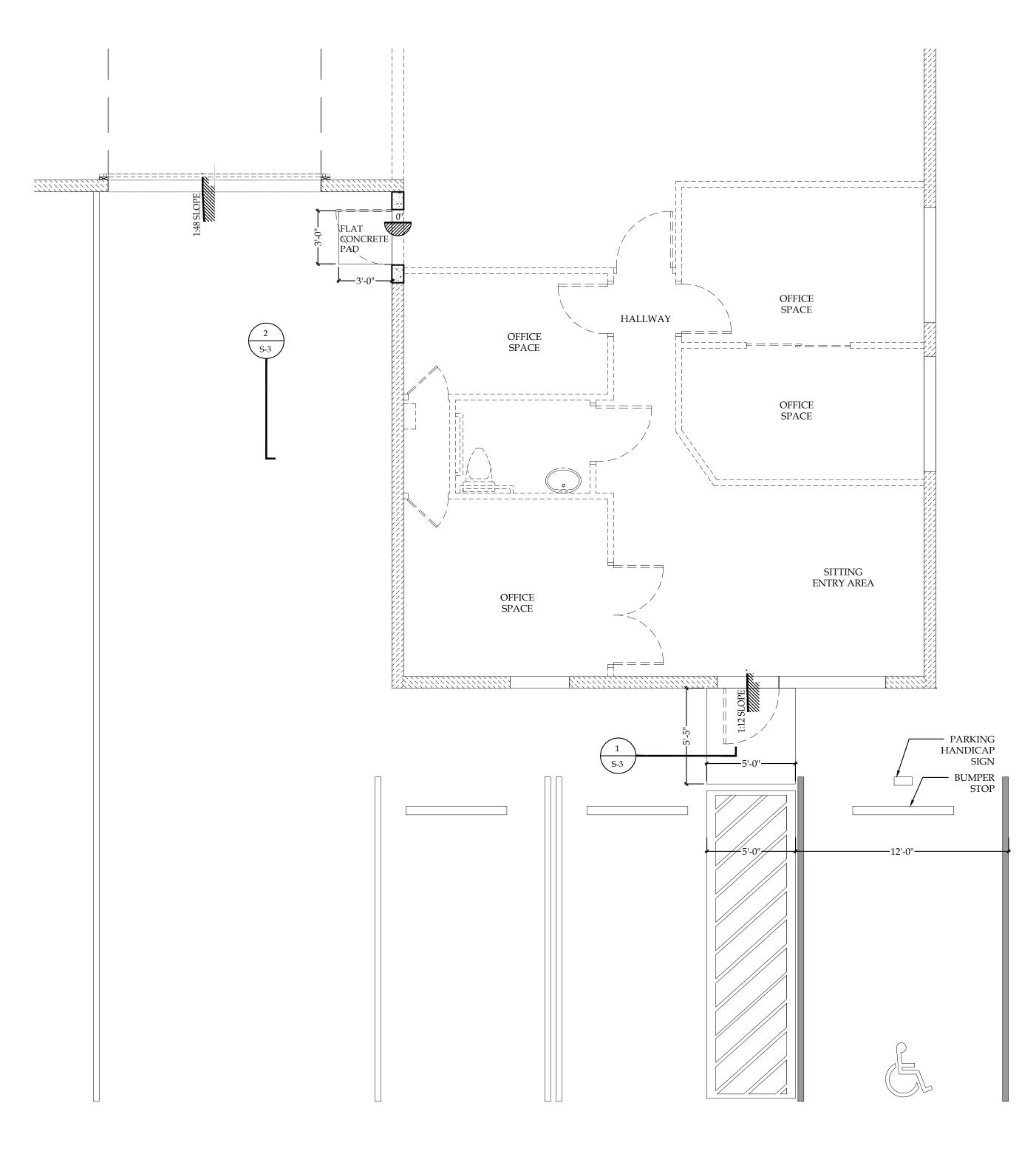




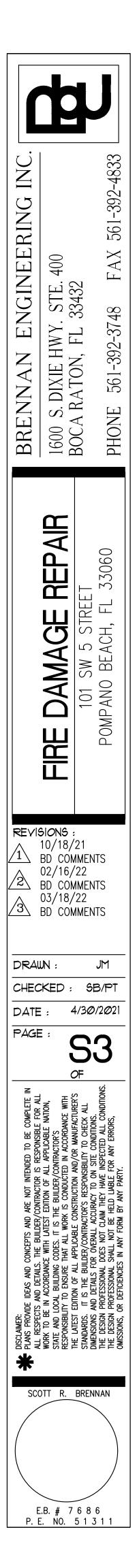
SOUTH ELEVATION DETAIL

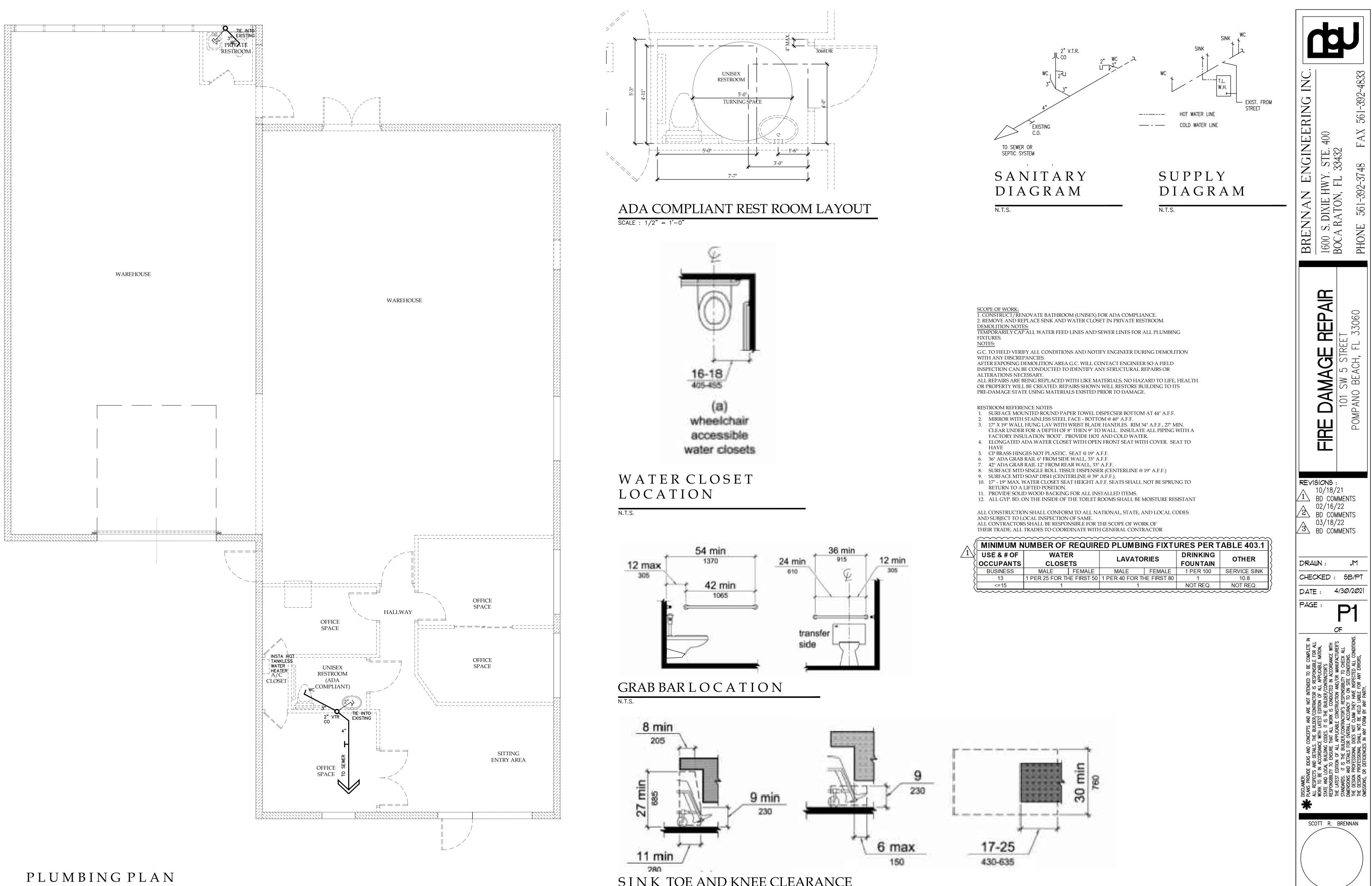
S-3 SCALE : $1/2^{"} = 1'-0"$ CONTRACTOR TO V.I.F. ALL MEASUREMENTS





PARTIAL PROPOSED PLAN SCALE : 1/4" = 1'-0" CONTRACTOR TO V.I.F. ALL MEASUREMENTS





SINK TOE AND KNEE CLEARANCE

E.B. # 7686 P.E. NO. 51311

N.T.S.

