

WORKSHEET
FOR HURRICANE
RESISTANT RESIDENTIAL
CONSTRUCTION

SSTD 10-97

CONDITIONAL PLAN APPROVAL
MASONRY **BUILDING** **SITE**



This plan is conditionally approved. Deficiencies must be identified and must be corrected by the applicant with applicable codes, state, county and city regulations or prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval.

This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code

This worksheet shall be used in conjunction with Southern Building Code Congress International "Hurricane Resistant Residential Construction Standard" SSTD 10-97

NOTE: The user is cautioned that this worksheet is not a substitute for compliance with the provisions contained in SSTD 10-97 or other applicable code requirements.

CHECKLIST FOR BUILDINGS WITH MASONRY EXTERIOR WALLS

102.1 BUILDING GEOMETRY

Number of Stories.....	1
Building Width (12 ft - 60 ft 1 story, 18 - 60, multistory).....	W = 37
Building Length.....	L = 58
Length to Width Ratio (L/W).....	L/W = 1.57
Building Height (30 ft maximum eave ht).....	H = 21
Ceiling Height (20 ft maximum).....	10
Roof Type (Gable or Hip).....	Hip
Roof Pitch (2:12 - 12:12).....	6:12
Roof Overhang at Sidewalls (4 ft maximum).....	2.00
Rake Overhang at Gable Endwalls (12 in maximum).....	

105 NONRECTANGULAR BUILDINGS (Add Leg Dimensions)

Number of Stories.....	1
Building Width (12 ft - 60 ft 1 story, 18 - 60, multistory).....	W = 24
Building Length.....	L = 28
Length to Width Ratio (L/W).....	L/W = 1.17
Building Height (30 ft maximum eave ht).....	H = 18
Ceiling Height (20 ft maximum).....	8
Roof Type (Gable or Hip).....	HIP
Roof Pitch (2:12 - 12:12).....	6:12
Roof Overhang at Sidewalls (4 ft maximum).....	2.00
Rake Overhang at Gable Endwalls (12 in maximum).....	

104.3 DESIGN WIND SPEED, mph. Wind Speed = 100

102.2 FOUNDATION TYPE (check appropriate type):

1. Stemwall Foundation w/ Slab-On-Grade (3 ft high max).....	
2. Stemwall Foundation w/ Crawl Space (3 ft high max).....	
3. Monolithic Slab-On-Grade.....	X
4. Pile Foundation (requires engineering design).....	

202.1 MATERIALS

Concrete Masonry Units (ASTM C 90 or C 145, 1900 psi min.).....	
Type M or S mortar 2150 psi min. Type N mortar.....	
Clay Masonry Units (ASTM C 82, C 216 or C 652 H40V, 4400 psi min.).....	X
Type M or S mortar, 5000 psi min, Type N mortar.....	
Mortar (Type M, S, or N, ASTM C 270).....	X
Grout (3/6 in max aggregate, 8-11 in slump, 2000 psi or ASTM C 476).....	
Concrete (2500 psi minimum compressive strength).....	X
Reinforcing Steel (Grade 40 minimum).....	X
Corrosion Protection for Metal Accessories (galvanized).....	ASTM A
Corrosion Protection for Fasteners and Connectors.....	Grade 40
1. Exposed to weather (stainless steel or hot dipped galv.).....	GAL.
2. Coastal area, salt air exposure (stainless steel or hot dipped galv.).....	

203 FOOTINGS AND FOUNDATIONS

Table 203A Stemwall Footing w/2 #5.....	T	W
Monolithic Slab-On-Grade-Exterior w/2 #5.....		
Monolithic Slab-On-Grade-Interior w/2 #5.....	T	16 W 12
203.3 Footing Dowels to Match Wall Reinforcement with 90 degrees hook, 5 inch or 6 inch min embedment, and 25 inch lap at:.....	T	W
1. Building corners.....		X
2. Each side of openings more than 6 ft wide.....		X
3. Ends of each shear wall segment.....		X
4. Other required vertical wall reinforcement of buildings located in 110 mph zone.....		
5. Other required vertical wall reinforcement in walls of buildings wider than 40 ft and located in 100 mph zone.....		6' & 8" O.C
6. Required vertical reinforcement in exterior walls where aggregate area of openings exceeds 25% of wall area.....		

CHECKLIST FOR BUILDINGS WITH MASONRY EXTERIOR WALLS

204 FLOOR SYSTEMS

- 204.1 Suspended Concrete Slabs (hollowcore per manuf. design).....
- 204.2 Monolithic Slab-On-Grade (3 1/2 inch thick mm., no reinforcement req'd). 4" SLAB

204.3 WOOD FRAME

- Floor Joists (sized per AF&PA span tables).....
- Floor Trusses (designed per TPI spec).....
- Floor Sheathing (7/16-inch wood structural panels).....
- Floor sheathing spans (per Table 2307.6B of SBC).....
- Bracing (4 ft o.c. first two framing spaces each end of floor).....
- Sheathing fasteners (2306 SBC).....
- Connection to Masonry Wall (pressure treated ledger beam).....
- (optional stemwall connection per Figure 203D).....
- Floor Diaphragm (check capacity with Tables 204G and 204H).....
- Shear capacities of diaphragm assemblies (Tables 304C1 and 304C2).....

205 MASONRY WALLS

- 205.1 6-inch Thick (1-story W/10ft max. clg. ht. or top story W/8ft max. clg. ht.)... X
- 8-inch Thick (All others):.....

205.2 BOND (TIE) BEAMS (at top, each floor and gable):

- Table 2050 6-in walls: 6x8 high masonry or cip concrete
- Table 205D 6x12 high masonry or cip concrete
- 6x16 high masonry or cip concrete
- 8-in walls: 8x8 high masonry or cip concrete
- 8x12 high masonry or cip concrete
- 8x16 high masonry or cip concrete

207.6.1(3) Approved Precast Bond Beam

205.3 VERTICAL REINFORCEMENT

- One #5 each comer.....
- One #5 each side of openings wider than 12 inches for concrete walls.....
- One #5 each side of openings wider than 6 ft for masonry walls.....
- Two #5 or one #7 (openings wider than 12 ft in 110 mph zone).....
- Two #5 or one #7 (openings wider than 12ft/bldgs wider than 40ft in 100mph zone).....
- One #5 where girders or girder trusses bear on masonry walls.....
- Wall Spacing per Tables 205C and 205D.....
- Shear Wall Spacing per 205.5.5.....

205.4 CONTINUOUS MASONRY GABLE

- Rake Beam: cip concrete, 4-in high min. w/1 #5.....
- 2x nailer bolted to rake beam.....
- Alternate ceiling diaphragm per 206.....

205.5 EXTERIOR SHEARWALLS

- Required shearwall length at endwalls (Table 205H)..... 1.8
- Required shearwall length at sidewalls (Table 205G)..... 1.8
- Maximum distance between shearwalls = 2.5 x bldg. width..... 93
- Minimum shearwall length = 2-ft or 4 ft per 205.5.1 CH/2 = 4
- Sum of shearwall segments per 205.5.2.....
- Shearwall segments connected by bond beam..... X
- Shearwall openings: 5 in. for piers and 12 in. above and below piers max dimension, 144 in2 max.....
- Shearwall reinforcing per 205.5.1..... X

205.6 INTERIOR SHEARWALLS

- When used, can decrease length/width ratio..... New L/W =
- Interior bond beam full width of building
- Top of shearwall supported per Figure 207H

205.7 CONTINUITY OF VERTICAL WALL REINFORCEMENT

- Minimum lap splices: #5 to #5=25 inches X
- #7 to #7=35 inches.....
- One #5 to one #7 = 25 inches.....
- two #5 to one #7 = 35 inches.....
- Standard hook embedded 6 inches into bond beam: 10-in leg for #5..... X
- 14-in leg for #7.....

CHECKLIST FOR BUILDINGS WITH MASONRY EXTERIOR WALLS

205.8 ASSEMBLIES AND BEAMS SPANNING OPENINGS

Pre-engineered assemblies for masonry walls:	
Extend 4 inches past each side of opening.....	8"
Precast-bottom story and top story of 2 story, Table 205P1.....	X
Precast-bottom story of 2 story, second and bottom story of three-story building w/wood floor) Table 205P2.....	
Precast-bottom story of 2 story, second and bottom story of three-story building w/hollowcore floor, Table 205P3.....	
Continuous Bond Beam Acting as Lintel:	
1 story and top story of 2 story, Table 205R1.....	
Bottom story of 2 story, second and bottom story of three-story building w/wood floor, Table 205R2.....	
Bottom story of 2 story, second and bottom story of three-story building w/hollowcore floor, Table 205R3.....	
Bond Beam Combined with Lintel:	
1 story and top story of 2 story, Table 205S1.....	X
Bottom story of 2 story, second and bottom story of three-story building w/wood floor, Table 205S2.....	
Bottom story of 2 story, second and bottom story of three-story building w/hollowcore floor, Table 205S3.....	

206 CEILING SYSTEMS

206.1 Ceiling Diaphragms (req'd when endwall tops at ceiling)	
206.3 Gypsum ceiling diaphragm at sidewall.....	X
Gypsum ceiling diaphragm at endwall.....	Hip
Wood structural panel ceiling diaphragm, Table 206D.....	
Wood structural panel ceiling diaphragm at endwall.....	
Wood structural panel ceiling diaphragm at sidewall.....	

207 ROOF SYSTEMS

207.1 RAFTER-JOIST FRAMING SYSTEMS	
Rafters: sized per AF&PA span tables (24 in o.c. max.).....	
Ridge Board: 2x min. cut depth of rafter.....	
Collar Beam: 1x6 every third rafter pair.....	
207.2 TRUSS FRAMING SYSTEMS	
Truss design per TPI spec.....	
Designs to Indicate wind speed, height and uplift.....	
Maximum truss spacing at 24 inches.....	
Girder trusses designed as drag struts.....	
Step-down hip system used for hip roof.....	
207.3 ROOF SHEATHING	
15/32 Exposure 1 wood structural panel.....	
Typical fasteners: 8d common or galv. box at 6 inches throughout.....	
8d ring shank in roof zone 3 and 110 mph, and with Group III lumber in 100 mph zone.....	
4-in nail spacing at gable endwall/gable truss.....	X
12-in intermediate spacing in roof zone 1 and 90 mph.....	
12-in intermediate spacing using Group II lumber in 100 mph zone.....	X
Alter as required for stronger diaphragm per 207.5.....	
207.4 BRACING	
Add blocking at 4ft o.c. 1st 2 framing spaces if no ceiling diaphragms....	X
207.5 ROOF DIAPHRAGM	
Required diaphragm capacity from Tables 207C and 207D.....	114 128
Roof diaphragm selected (Tables 304C1 and 304C2).....	
Diaphragm nailing requirements (207.3.3, Tables 304C1 and 304C2).....	207.3.3

CONDITIONAL PLAN APPROVAL
BUILDING
 SITE

This plan is conditionally approved. The applicant must be responsible for obtaining all applicable codes and regulations of the jurisdiction where the project is located. The applicant must be connected to the water and sewer system. The applicant must obtain all necessary permits from the local, state, county or city government. If not be constructed as shown, alter or set aside.

207.6 CONNECTIONS FOR WOOD ROOF SYSTEMS

Sidewall, Truss/Rafter to Bond Beam:		
Connectors rated for uplift (Table 207E).....	650 MAX.....	TAP 18
Connectors rated for lateral load (207.6.1(3)).....		X
Sidewall, Bolted Top Plate Alternate:		
Bolt, washer, nut, top plate material per 207.6.2(1).....		
Bolt spacing (24 in @ 90mph, 21 in. @ 100mph, 18 in. @ 110mph).....		
9/16 inch max. dia. bolt hole in top plate.....		
Bolts 6 in. max. each side of plate splice.....		
Bolts 12 in. max. from end of plate.....		
Truss rafters fastened to top plate w/rated connector per 207.6.1.....		
Continuous Gable Endwalls:		
Pressure treated 2x nailer bolted to rake beam		
w/ 1/2-in anchor bolt spaced per Table 205E.....		
Gable Truss Endwalls (permitted only where clg. diaphragm needed).....		
Shear connector rated for diaphragm capacity		
in Table 207D times connector spacing (feet).....		2
OR 2x pressure treated plate bolted to bond beam		
(1/2-in dia. 04 ft o.c.).....		
Wood Framed Gable Endwalls: Refer to 403.....		
Hip Roof Trusses at Endwalls: Modify sidewall details using Table 207J...		1396
Interior Shearwall to Roof: Similar to endwalls.....		

208 OPEN STRUCTURES

208.1 GENERAL

Foundations: Same as I story building of same size.....		
Common Wall: #5 infilled cell at juncture.....		
Bond Beams/Lintels: Rated for loads of Tables 208A, 208C, 208E.....		
or from 205.8, Beams Spanning Openings.....		
Columns (max. 10 ft high to top of bond beam).....		
Corner Columns:.....	Size=	16 in. x 16 in.
Vert. Reint. (4 #3 for 8x8, 4 #5 all others).....		3 #5
Standard hooks, column to foundation.....		3 #5
Standard hooks, column to bond beam.....		3 #5
Column ties.....		# @
Intermediate Columns:.....	Size=	# @
Vert. Reint. (4 #3 for 8x8, 4 #5 all others).....		# @
Standard hooks, column to foundation.....		# @
Standard hooks, column to bond beam.....		# @
Column ties.....		# @

209 EXTERIOR WALL VENEERS

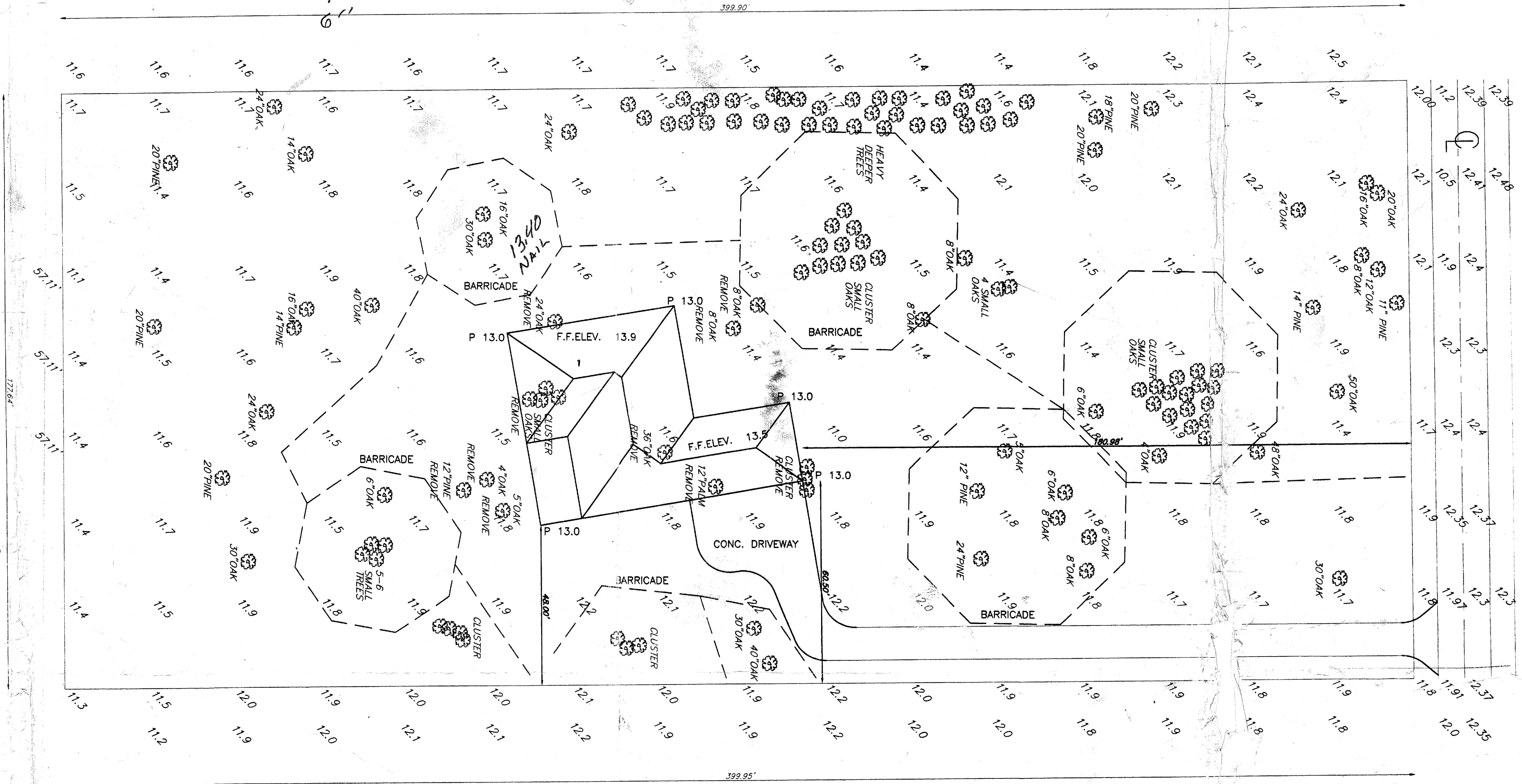
209.1 STUCCO (per ASTM C 926).....		X
209.2 BRICK VENEER (metal ties per Table 209A).....		

PRINT NAME CARLOS CASTILLA

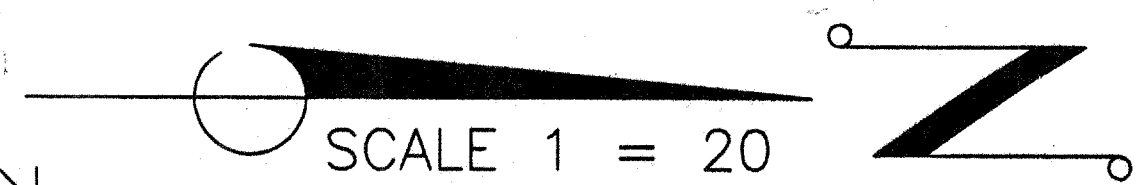
SIGNATURE *Carlos J. Castilla*

H & R R 15

23/3
6.2
10
6



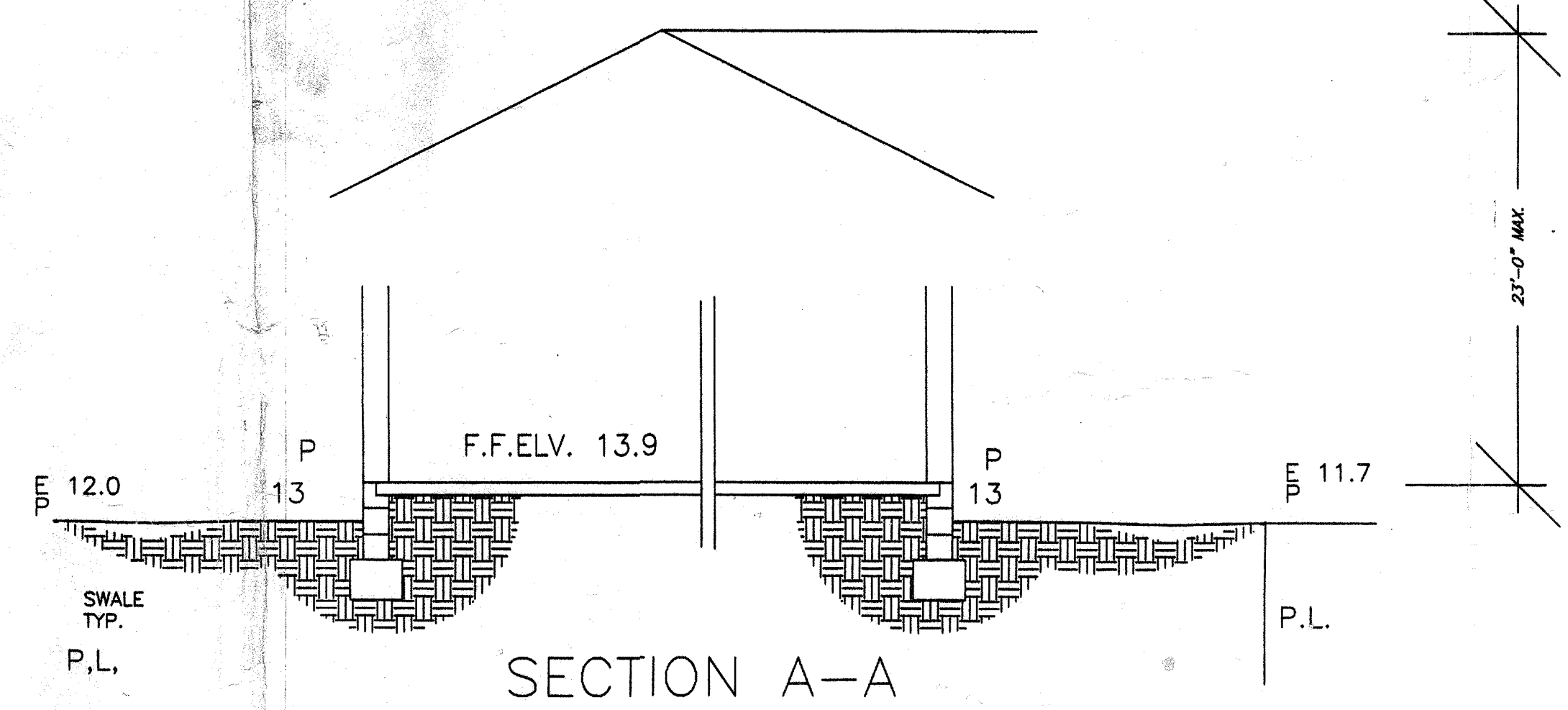
WEST ELROD AVENUE



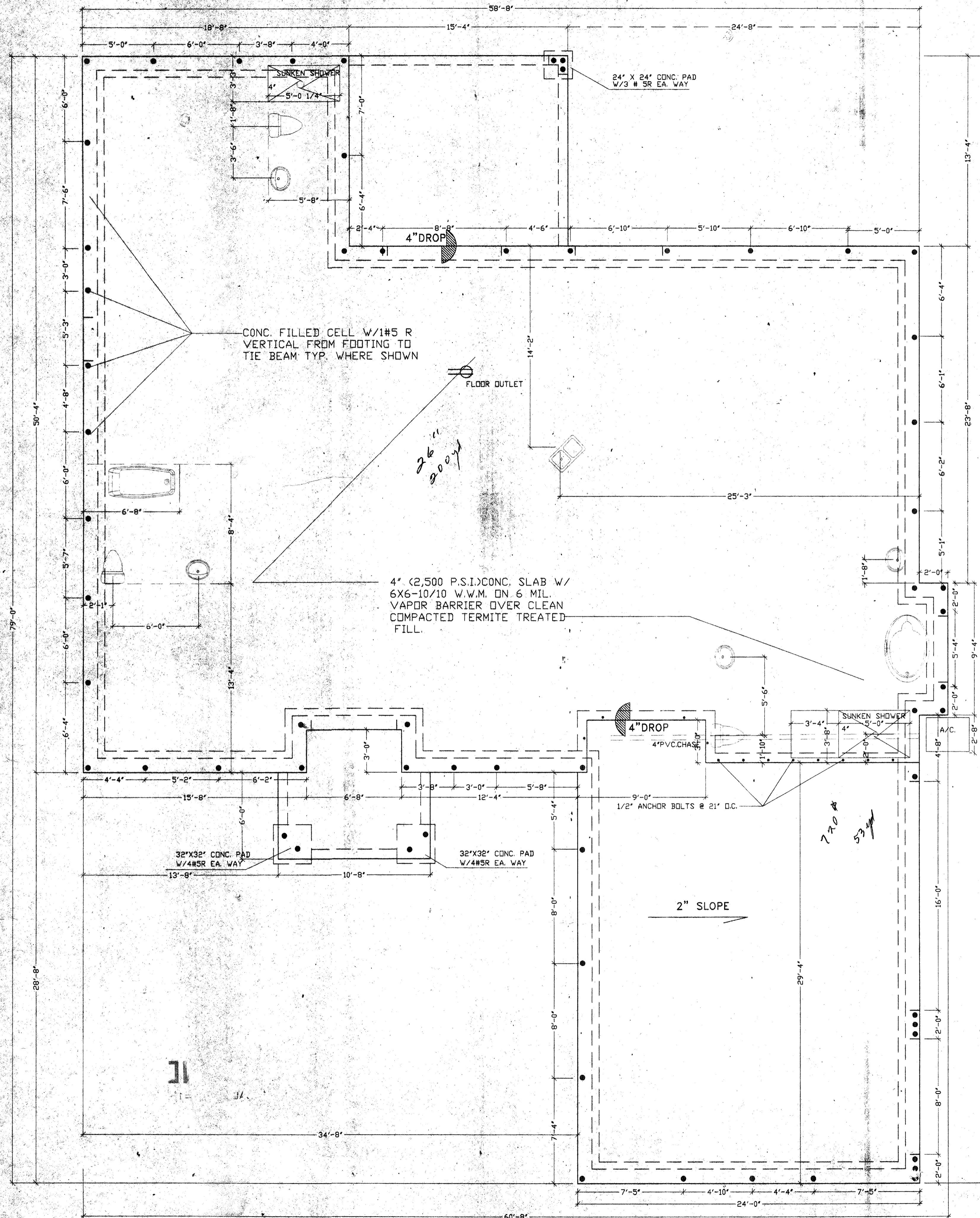
FOLIO # 179766.0000

CARLOS CASTILLA
265 8660
14914 GLASGOW CT.

LOT 12, LESS THE SOUTH 225.00 FEET, OF BLOCK 2 OF WALL'S SUBDIVISION ACCORDING TO A MAP OR PLAT THEREOF AS RECORDED IN PLAT BOOK 3, PAGE 69 OF THE PUBLIC RECORDS OF HILLSBOROUGH COUNTY, FLORIDA, TOGETHER WITH AN 8 FOOT WIDE EASEMENT, THE CENTERLINE OF WHICH IS DESCRIBED AS FOLLOWS: COMMENCING AT THE SOUTHWEST CORNER OF OF SAID LOT 12, THENCE EASTERLY ALONG THE SOUTH LINE OF SAID LOT 12 A DISTANCE OF 89.00 FEET TO THE POINT OF THE BEGINNING, THENCE NORTHERLY AND PARALLEL TO THE WEST LINE OF SAID LOT 12, A DISTANCE 225.00 FEET TO THE SOUTH LINE OF THE ABOVE DESCRIBED LAND AND POINT OF TERMINATION..... SAID EASEMENT FIRST GIVEN IN INSTRUMENT RECORDED IN O.R. BOOK 5309, PAGE 820 PUBLIC RECORDS OF HILLSBOROUGH COUNTY, FLORIDA. (DESCRIPTION FURNISHED BY TITLE CO.)



SECTION A-A



21/6/06
22/6/06
24/6/06
26/6/06

8
23/1
29/1

13/8
65/8

770 #
53/1

FOUNDATION PLAN

SCALE 1/4" = 1'-0"

CONDITIONAL PLAN APPROVAL

BUILDING SITE



This plan is conditionally approved. Deficiencies have been identified and corrections for compliance with applicable federal, state, county and city regulations or codes. These deficiencies must be corrected prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval.

This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.

REVISIONS:	
	REDRAW

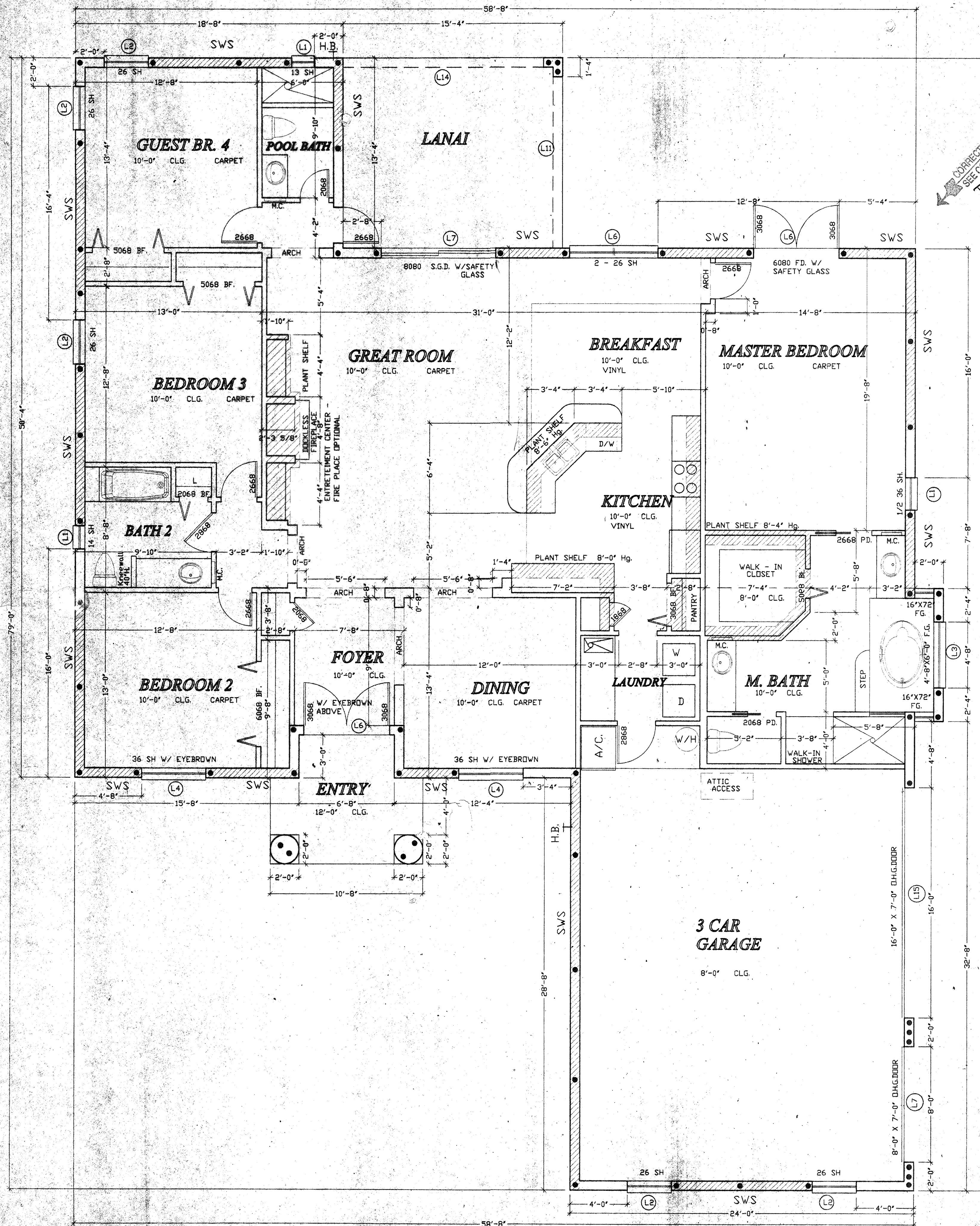
PROPOSED RESIDENCE FOR:
PRESTIGE EXECUTIVE HOMES
DIVISION OF ALL STATE HOMES INC.
Hillsborough County

ARCHITECTURAL
DESIGN-DRAFTING
CARLOS CASTILLA
813-265-8660 TAMPA FL. 2377

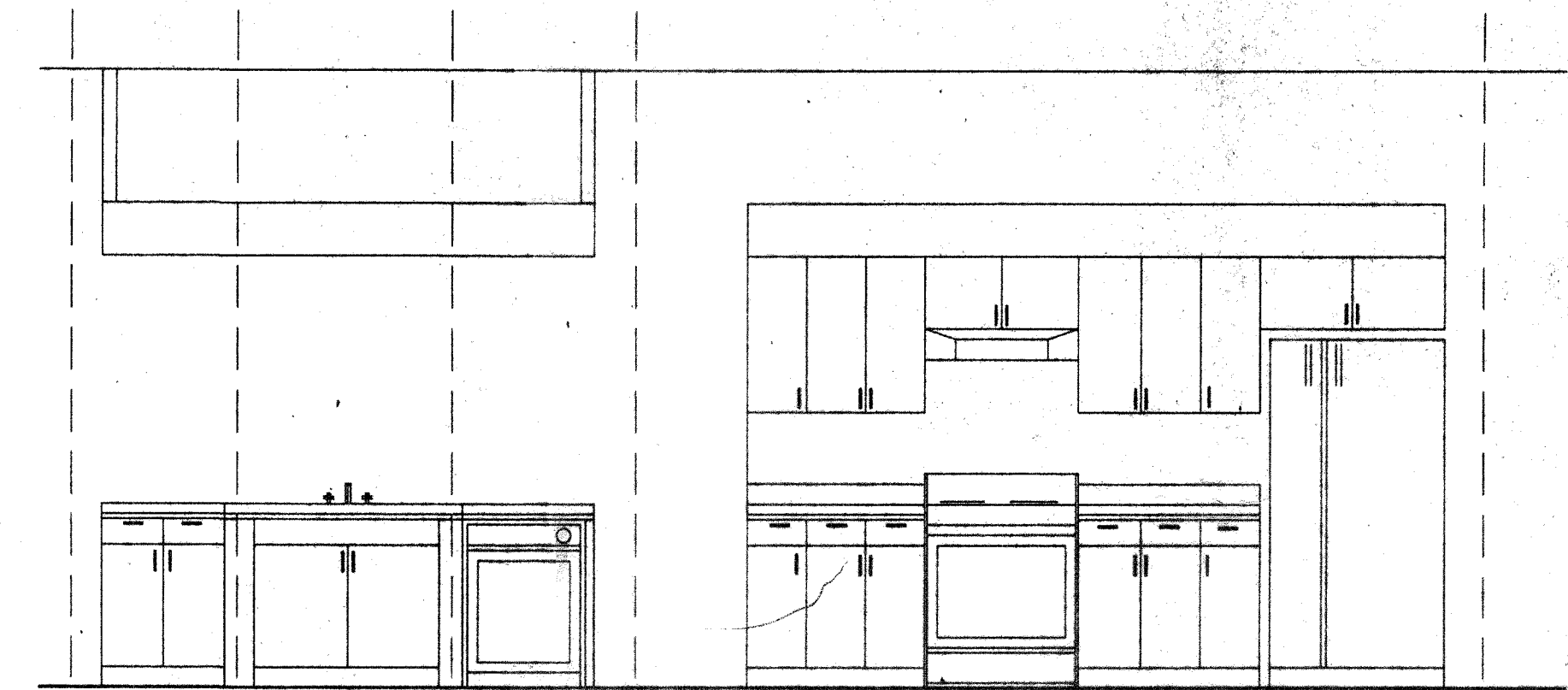
I HEREBY CERTIFY THAT I HAVE REVIEWED THE ATTACHED DESIGN AND FOUND IT TO BE IN COMPLIANCE WITH SECTION 1606 OF THE STANDARD BUILDING CODE AND / OR COMPLY WITH SSTD 1097

DATE: 6-3-99
JOB NO: HARRIS1
DESIGN BY: PL
DRAWN BY: CAC
CHECKED BY:

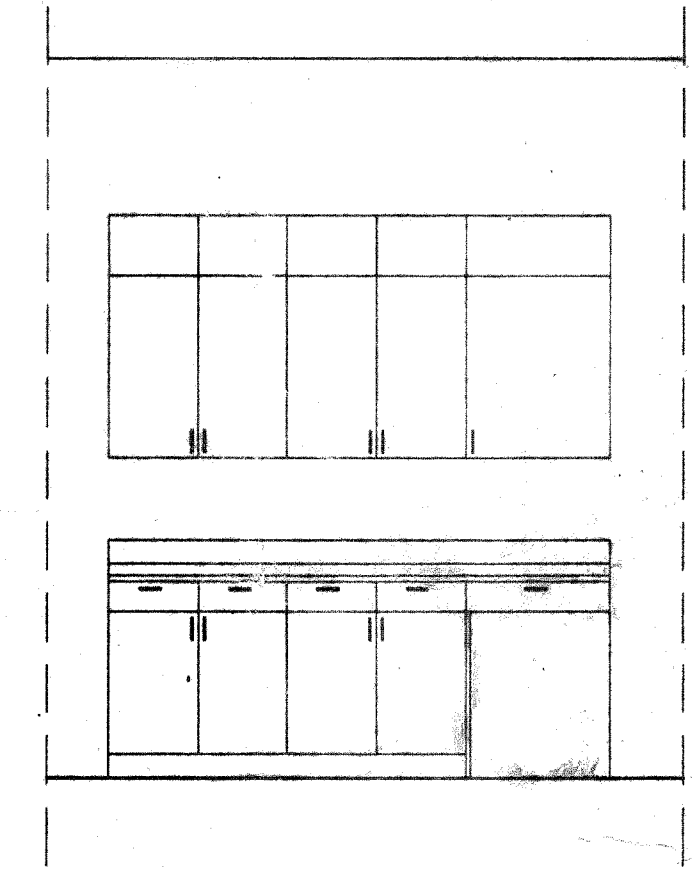
2036



NO CORRECTION IS REQUIRED
 SEE COMMENT SHEET ITEM
 TEMP CLASS NGT
 HAZARDOUS LOCATIONS



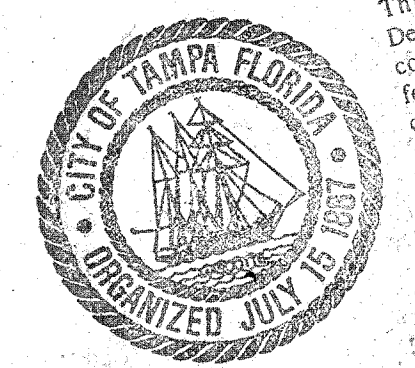
KITCHEN SCALE 3/8"=1'-0"



AREAS	
LIVING	2377
GARAGE	728
ENTRY	84
LANAI	533
TOTAL	3722

FLOOR PLAN
 SCALE 1/4"=1'-0"

CONDITIONAL PLAN APPROVAL
 BUILDING SITE
 This plan is conditionally approved. Deficiencies have been identified and must be corrected for compliance with applicable federal, state, county and city regulations or codes. These deficiencies must be corrected prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval. This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.



REVISIONS:
REDRAW

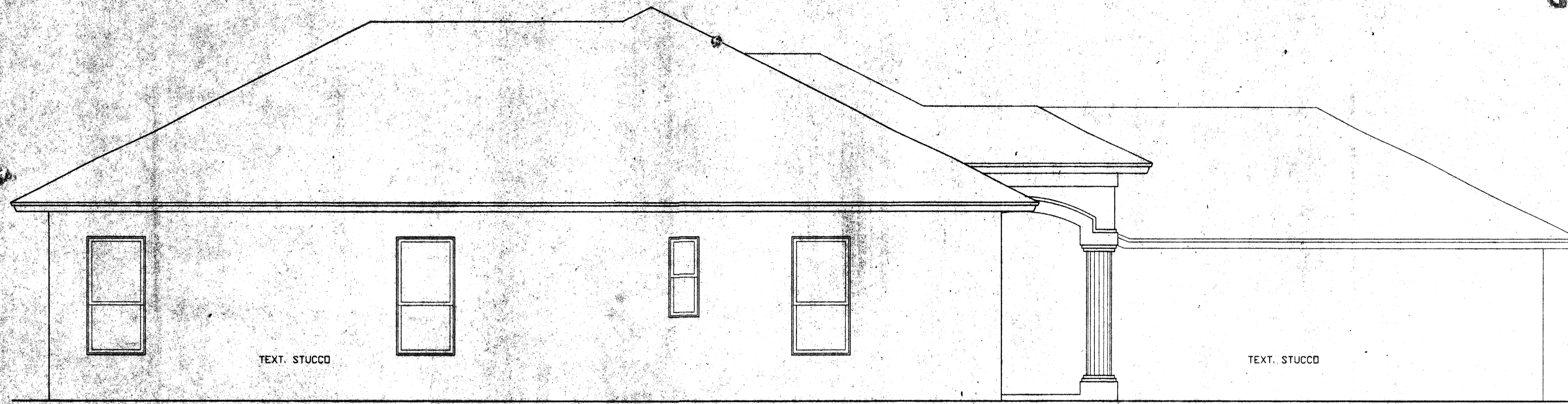
PROPOSED RESIDENCE FOR:
PRESTIGE EXECUTIVE HOMES
 DIVISION OF ALL STATE HOMES INC.
 Hillsborough County
 2386

ARCHITECTURAL
 DESIGN-DRAFTING
CARLOS CASTILLA
 813-265-8660 TAMPA FL

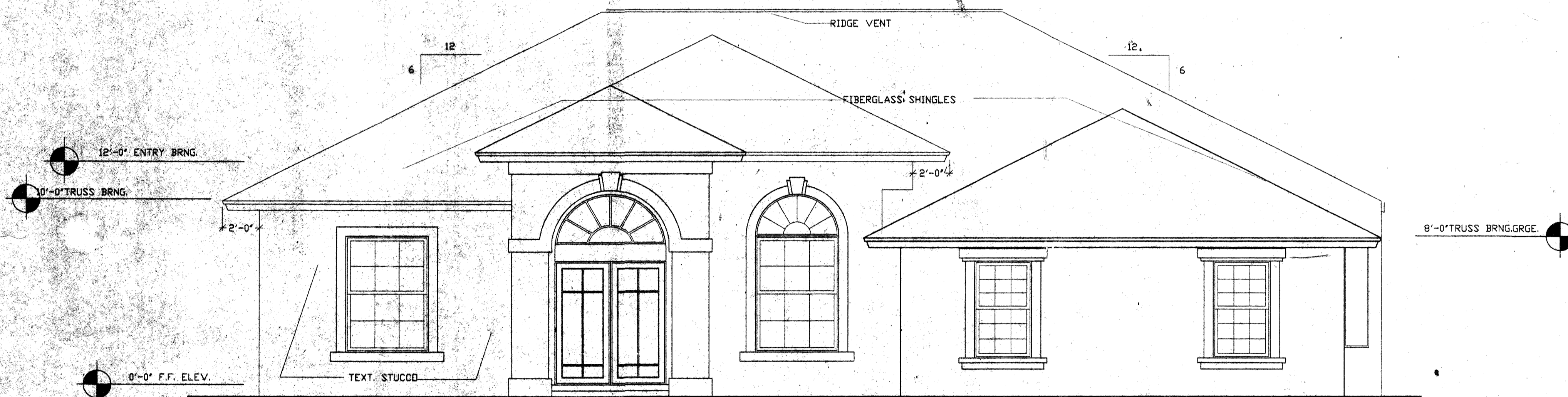
I HEREBY CERTIFY THAT I HAVE REVIEWED THE ATTACHED DESIGN AND FOUND IT TO BE IN COMPLIANCE WITH SECTION 1606 OF THE STANDARD BUILDING CODE, AND / OR COMPLY WITH SSTD 1097

DATE: 6-3-99
 JOB: NOHARRIS1
 DESIGN BY: PL
 DRAWN BY: CAC
 CHECKED BY:

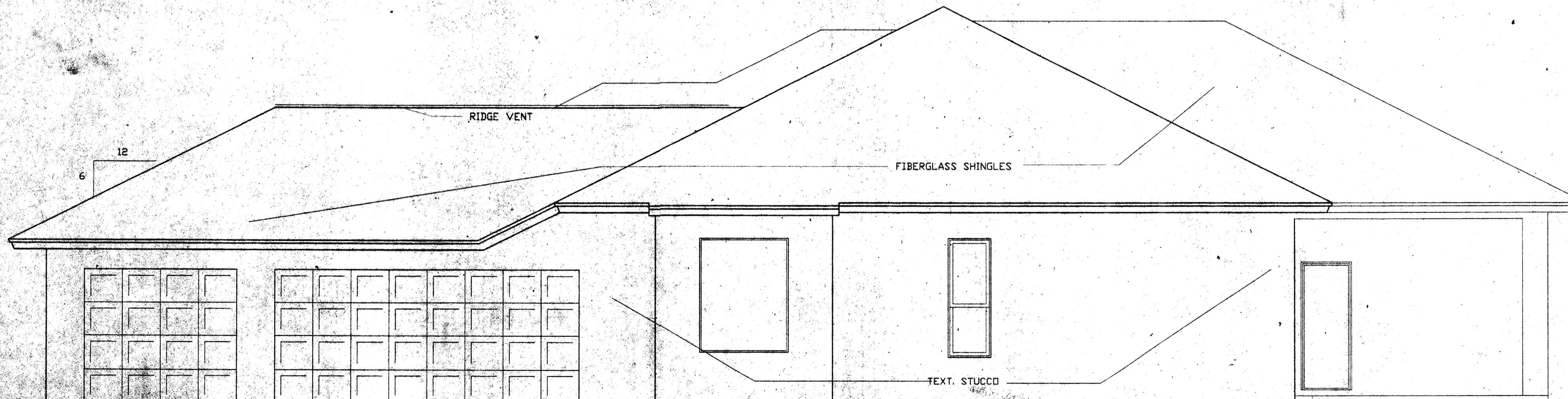
2036
 Sheet
2



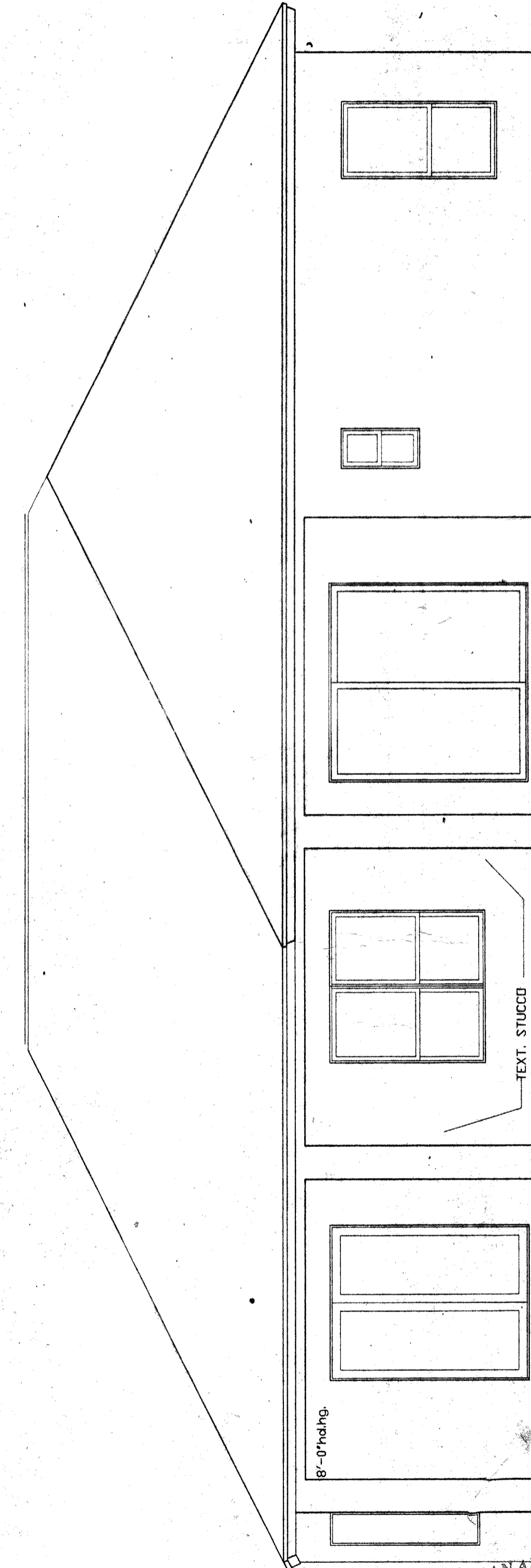
LEFT ELEVATION
SCALE = 1/4" = 1'-0"



FRONT ELEVATION
SCALE = 1/4" = 1'-0"



RIGHT ELEVATION
SCALE = 1/4" = 1'-0"



REAR ELEVATION
SCALE = 1/4" = 1'-0"



CONDITIONAL PLAN APPROVAL
 BUILDING SITE

This plan is conditionally approved. The applicant is responsible for obtaining all necessary permits and approvals from the applicable federal, state, and city regulations. This drawing is not to be used for construction without the approval of the City of Tampa. No liability is assumed in the issuance of this conditional approval.

This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.

REVISIONS:
REDRAW

PROPOSED RESIDENCE FOR:
PRESTIGE EXECUTIVE HOMES
DIVISION OF ALL STATE HOMES INC.
Hillsborough County

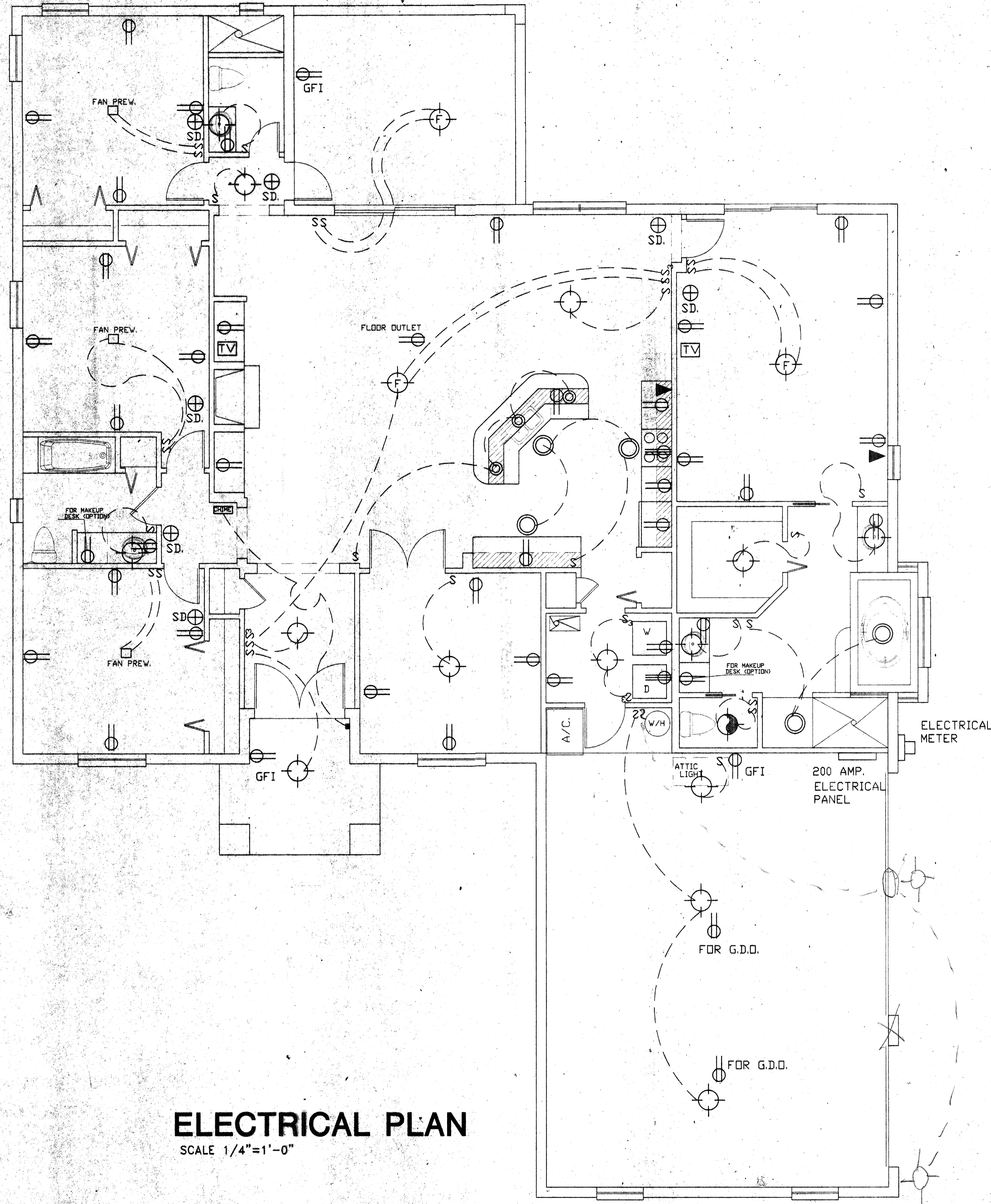
ARCHITECTURAL
DESIGN-DRAFTING
CARLOS CASTILLA
813-265-8660 TAMPA FL. 2377

I HEREBY CERTIFY THAT I HAVE REVIEWED THE ATTACHED DESIGN AND FOUND IT TO BE IN COMPLIANCE WITH SECTION 1606 OF THE STANDARD BUILDING CODE AND / OR COMPLY WITH SSTO 1097

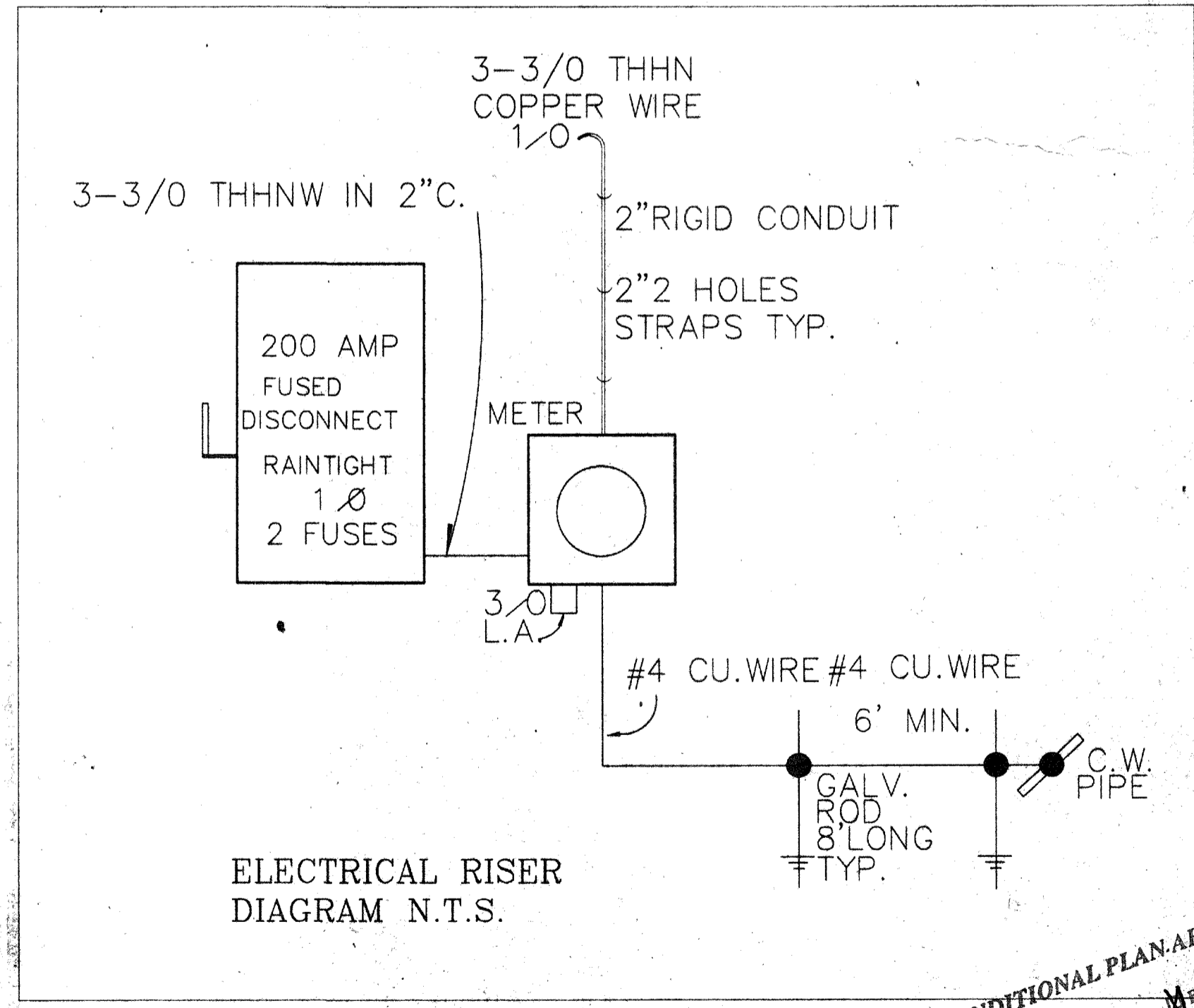
DATE: 6-3-99
JOB NO: HARRIS1
DESIGN BY: PL
DRAWN BY: CAC
CHECKED BY:

2036

Sheet 3



- OUTLET
- LIGHT
- LIGHT SWITCH
- PHONE
- TELEVISION
- SMOKE DETECTOR
- 220 OUTLET
- CHIME



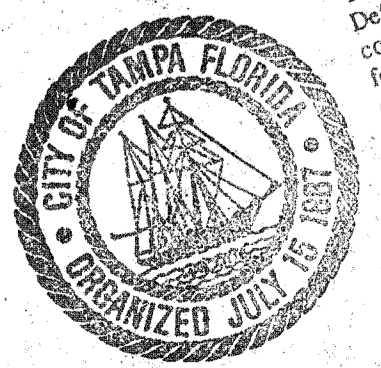
ELECTRICAL PLAN
SCALE 1/4"=1'-0"

CONDITIONAL PLAN APPROVAL

BUILDING SITE

This plan is conditionally approved. Deficiencies have been identified and must be corrected for compliance with applicable federal, state, county and city regulations or codes. These deficiencies must be corrected prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval.

This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.



REVISIONS:
REDRAW

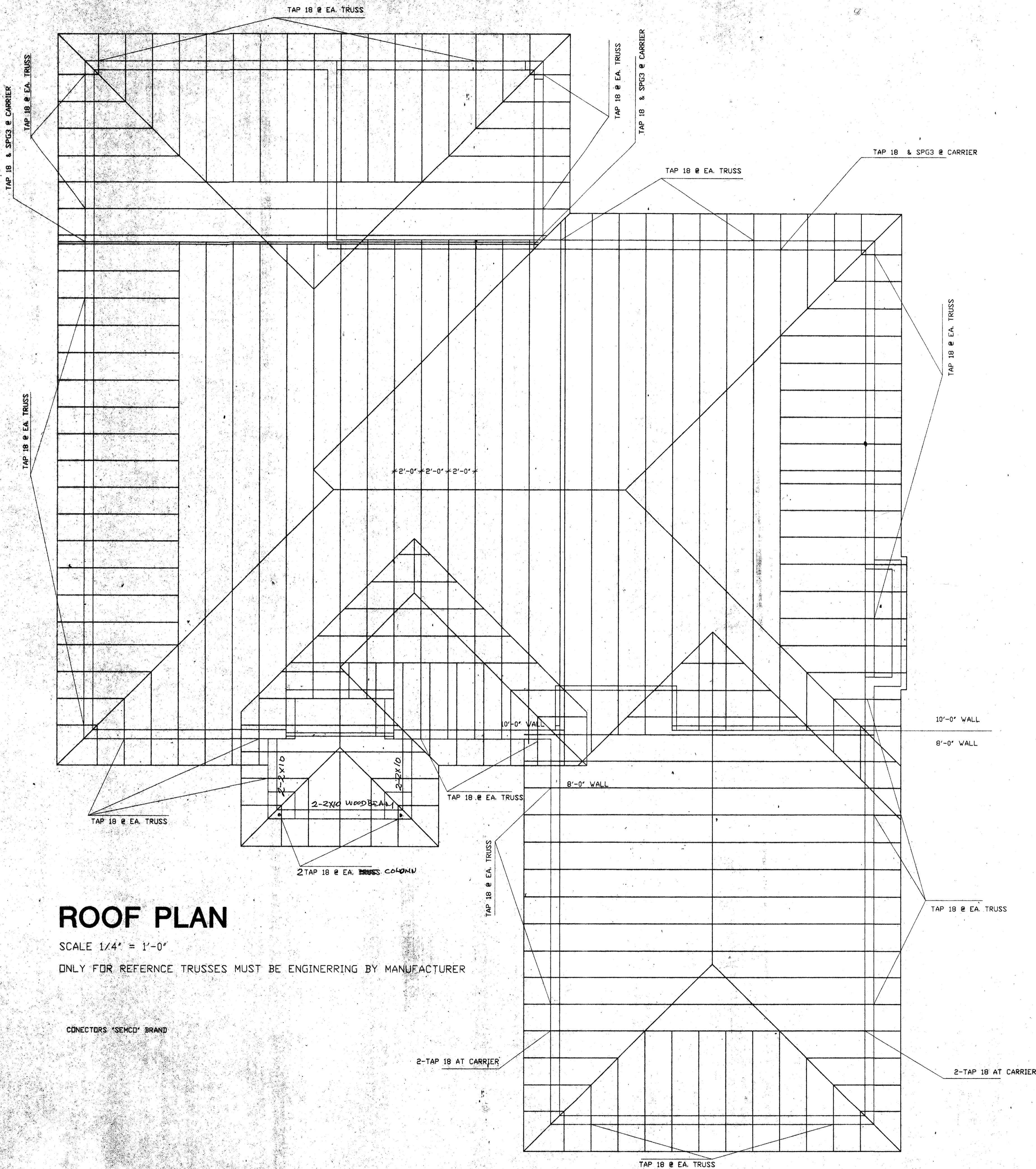
PROPOSED RESIDENCE FOR:
PRESTIGE EXECUTIVE HOMES
DIVISION OF ALL STATE HOMES INC.
Hillsborough County

ARCHITECTURAL
DESIGN-DRAFTING
CARLOS CASTILLA
813-265-8660 TAMPA FL.

I HEREBY CERTIFY THAT I HAVE REVIEWED THE ATTACHED DESIGN AND FOUND IT TO BE IN COMPLIANCE WITH SECTION 1606 OF THE STANDARD BUILDING CODE, AND / OR COMPLY WITH SSTD 1097

DATE: 6-3-99
JOB NO: HARRIS1
DESIGN BY: PL
DRAWN BY: CAC
CHECKED BY:

2036
Sheet 4

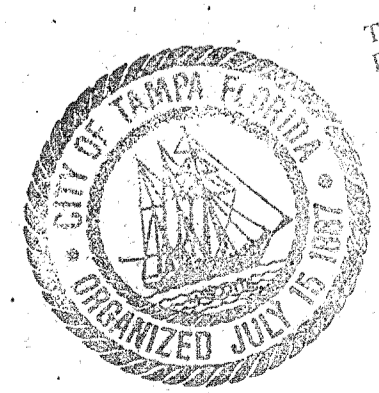


ROOF PLAN

SCALE 1/4" = 1'-0"

ONLY FOR REFERENCE TRUSSES MUST BE ENGINEERING BY MANUFACTURER

CONNECTORS 'SEMCO' BRAND



CONDITIONAL PLAN APPROVAL BUILDING SITE

This plan is conditionally approved. Deficiencies have been identified and must be corrected for compliance with applicable federal, state, county and city regulations or codes. These deficiencies must be corrected prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval. This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.

REVISIONS:
REDRAW

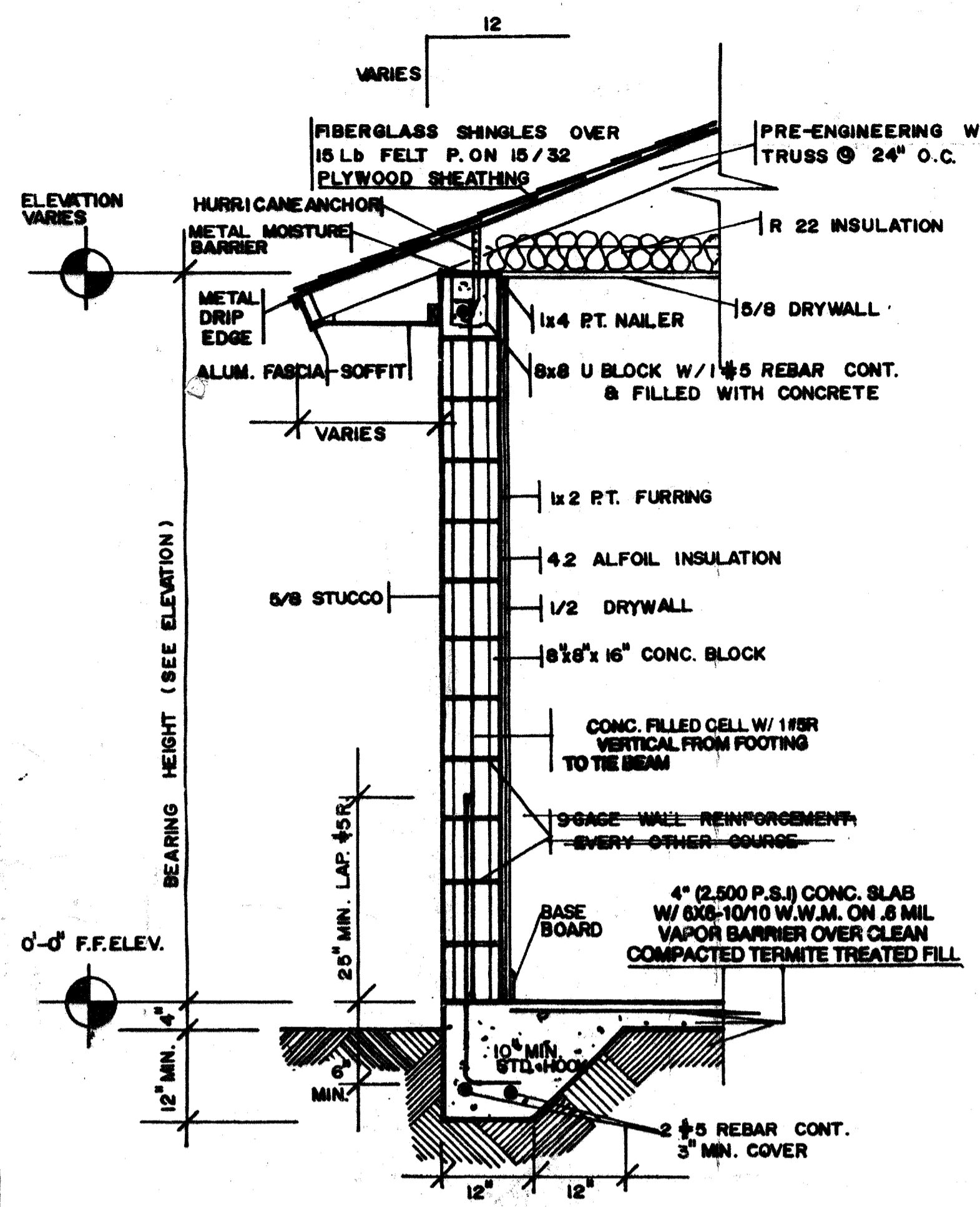
PROPOSED RESIDENCE FOR:
PRESTIGE EXECUTIVE HOMES
 DIVISION OF ALL STATE HOMES INC.
 Hillsborough County

ARCHITECTURAL
 DESIGN-DRAFTING
 CARLOS CASTILLA
 813-265-8660 TAMPA FL.
 2377

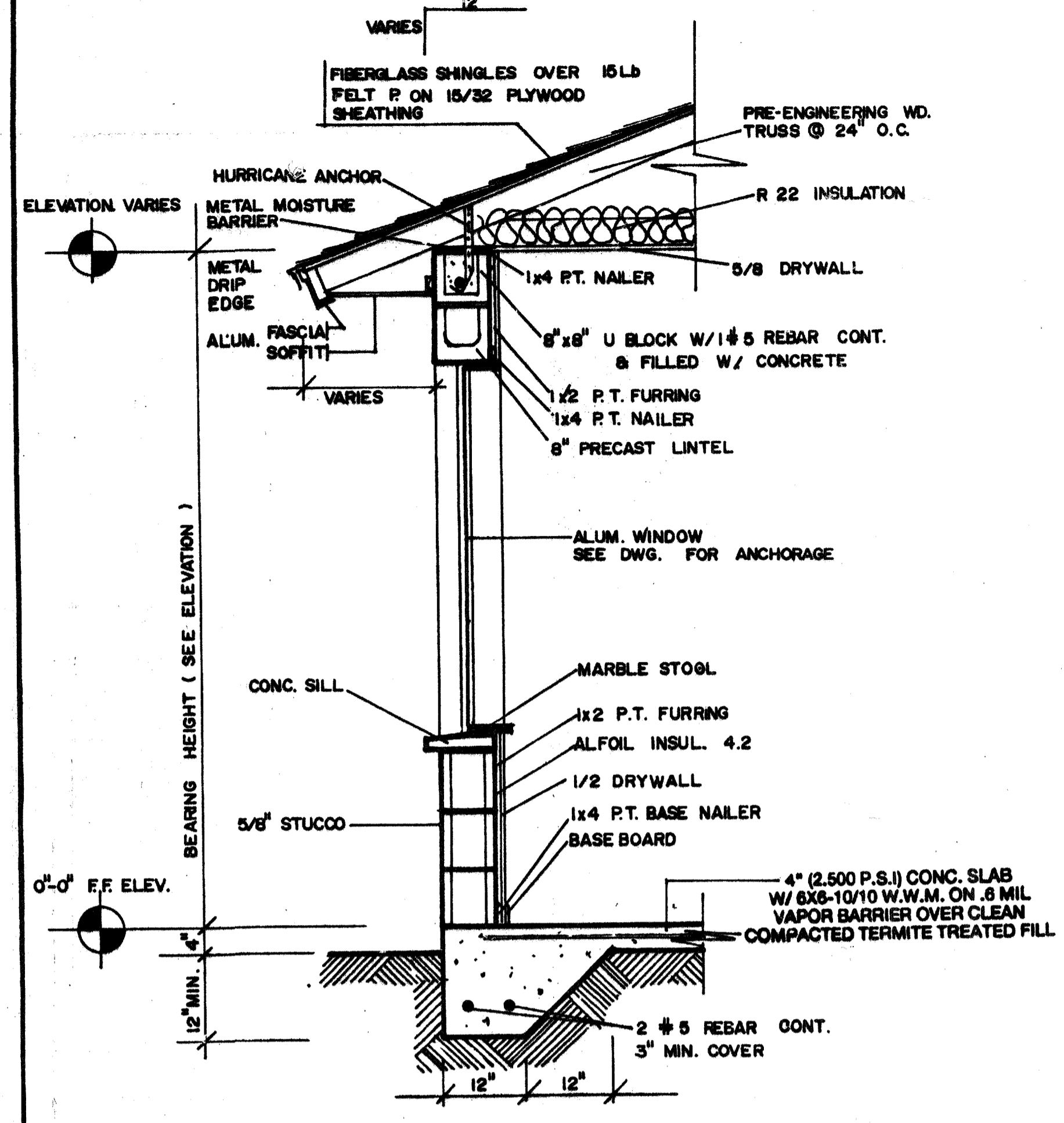
I HEREBY CERTIFY THAT I HAVE REVIEWED THE ATTACHED DESIGN AND FOUND IT TO BE IN COMPLIANCE WITH SECTION 1606 OF THE STANDARD BUILDING CODE. AND / OR COMPLY WITH SSTD 1097

DATE: 6-3-99
JOB NOHARRIS1
DESIGN BYPL
DRAWN BYCAC
CHECKED BY:

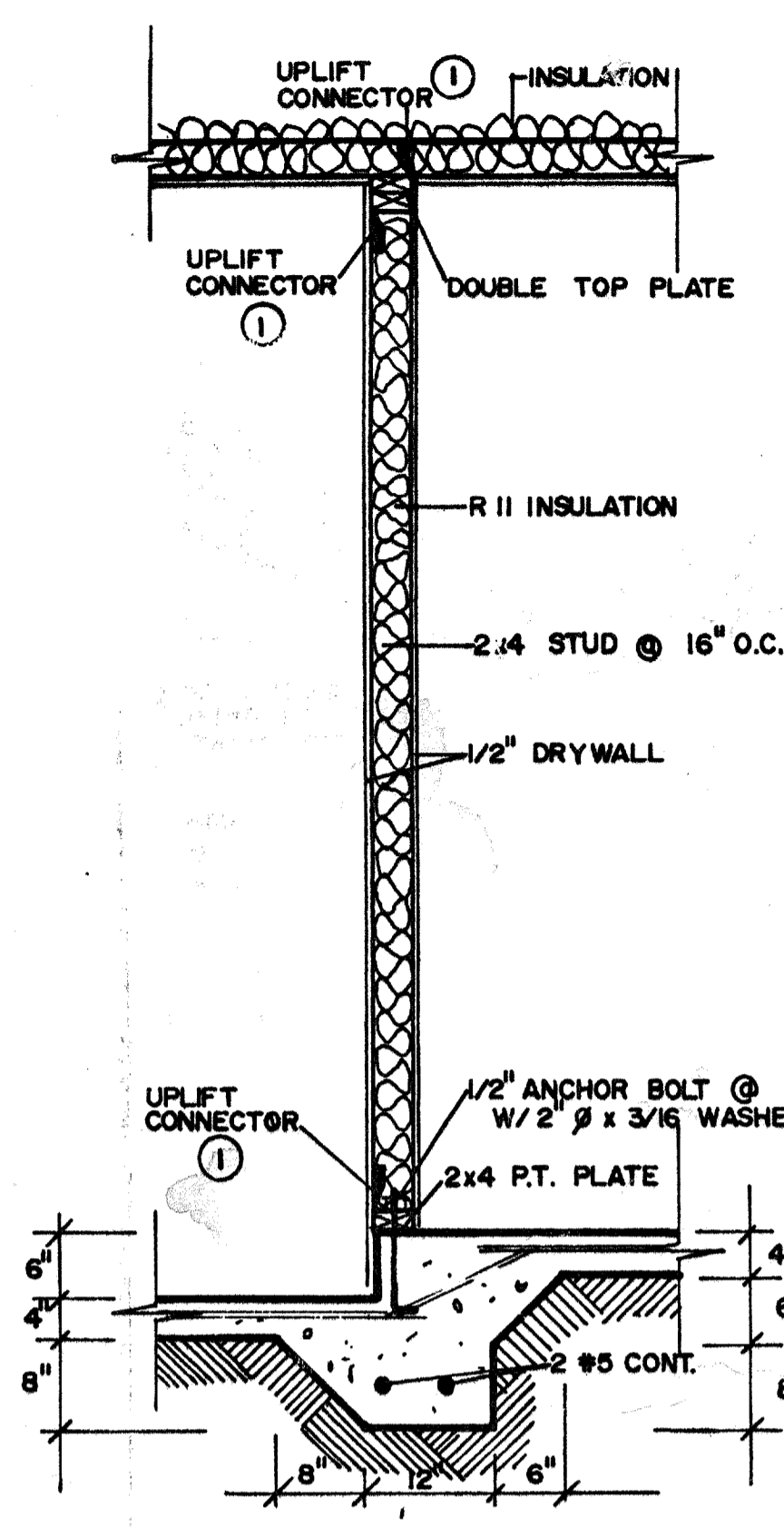
2036
 Sheet 5



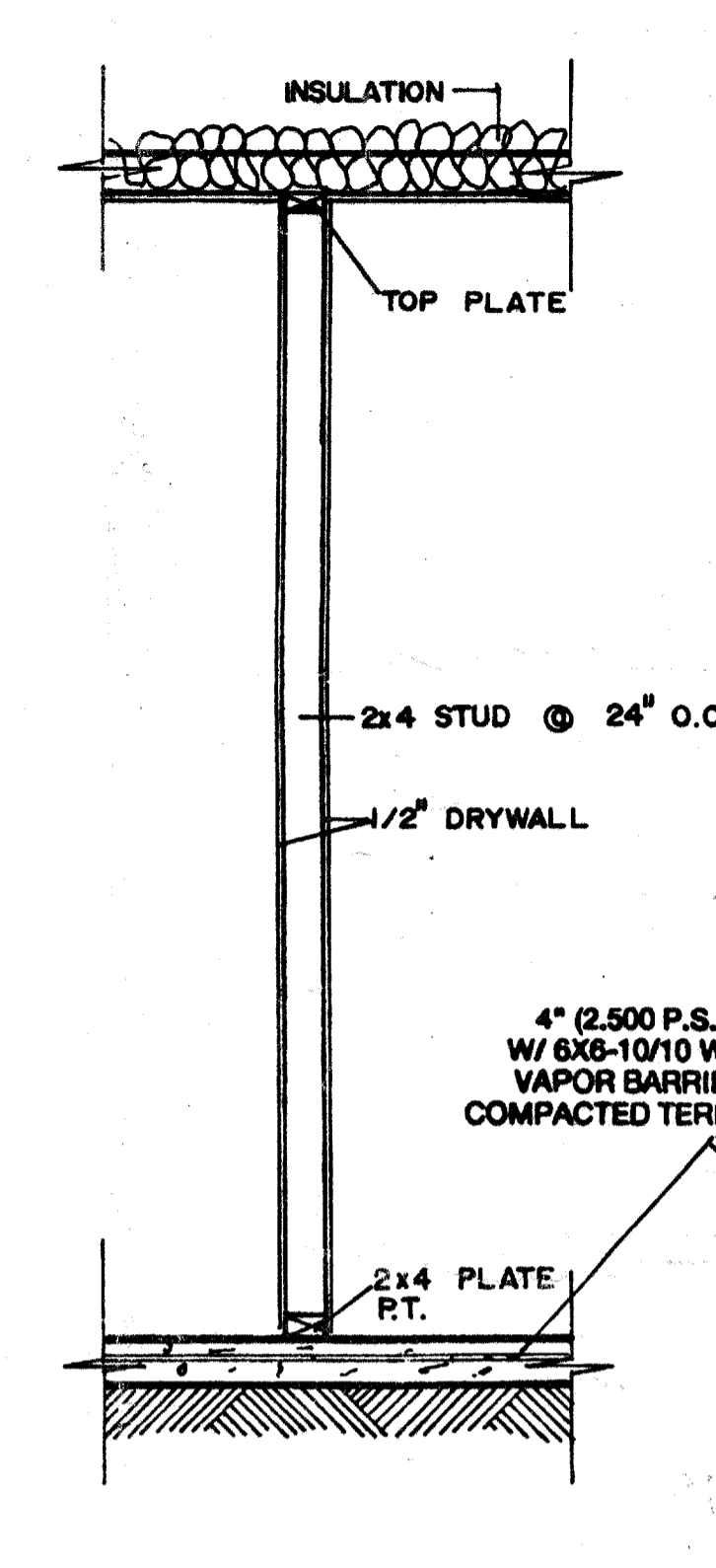
WALL SECTION SCALE 3/4"=1'-0"



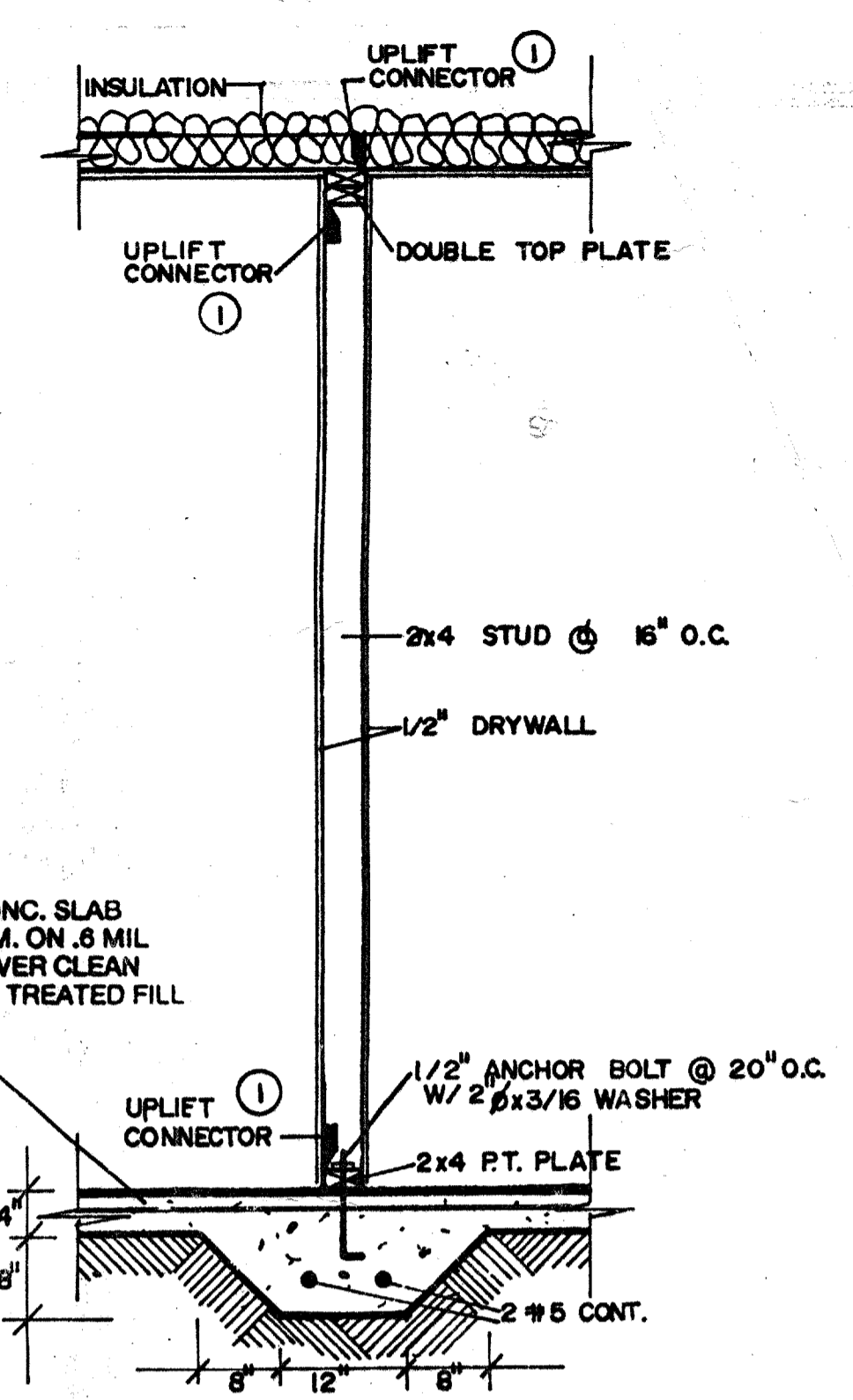
WALL SECTION THROUGH WINDOW SCALE 3/4"=1'-0"



INTERIOR BEARING WALL AT GARAGE



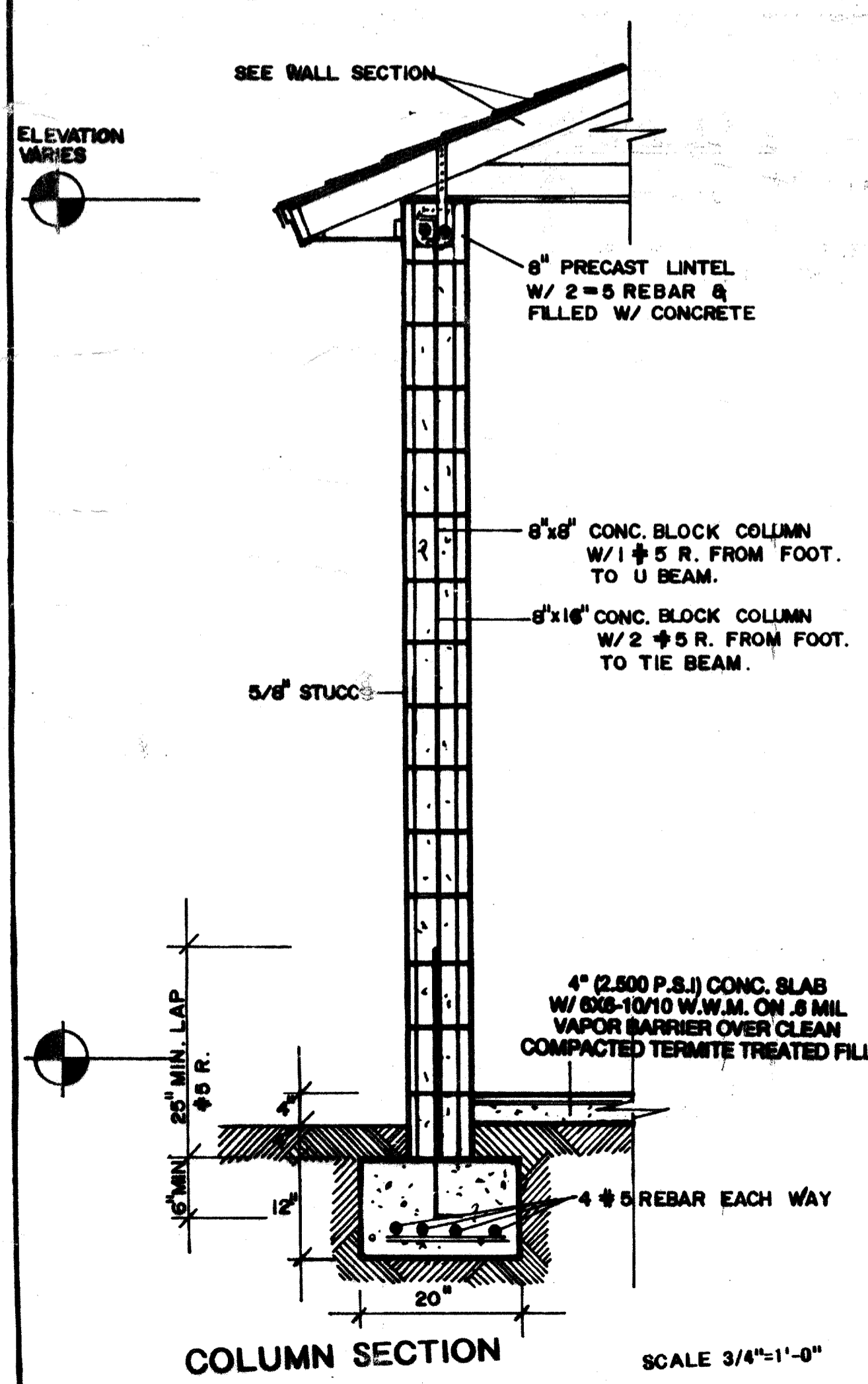
PARTITION WALL



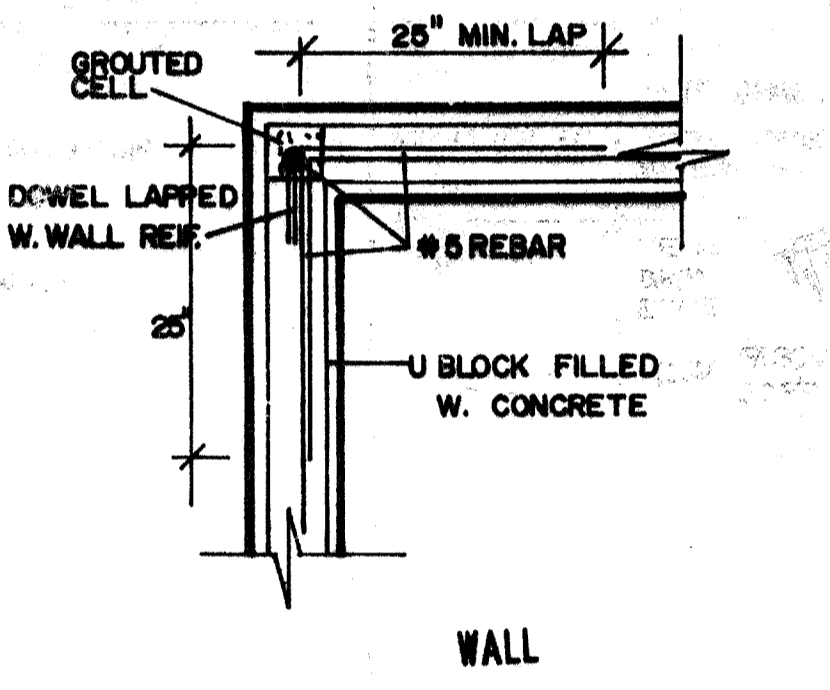
INTERIOR BEARING WALL

① = SIMPSON H3, SEMCO HCP R/L, OR HUGHES HC R/L H18

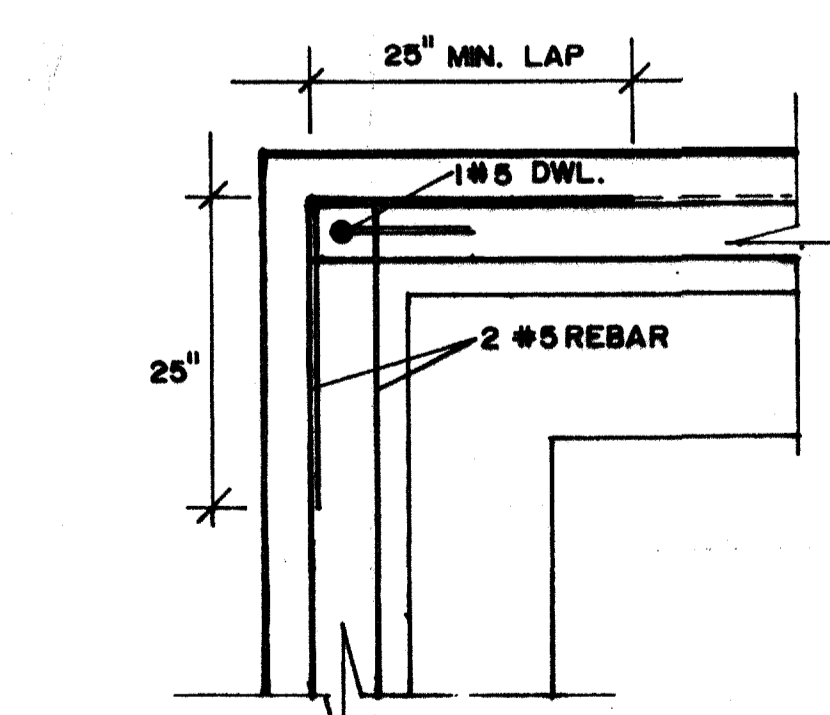
SCALE 3/4"=1'-0"



COLUMN SECTION SCALE 3/4"=1'-0"

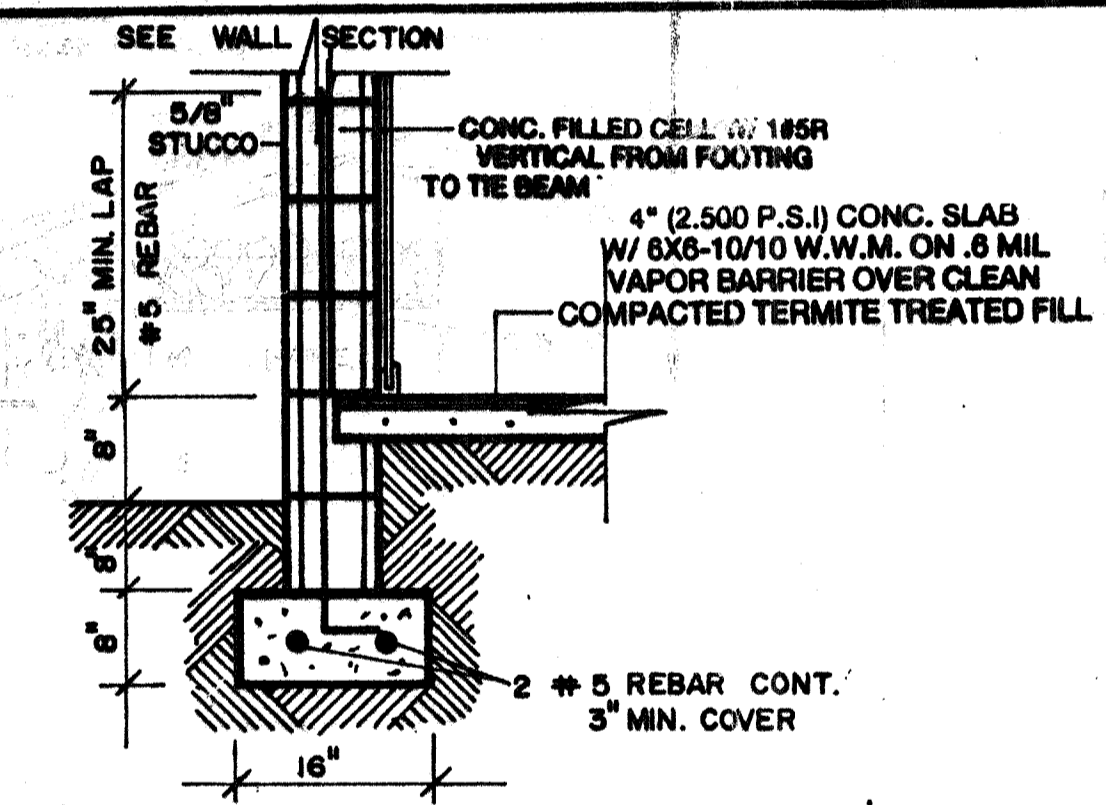


WALL

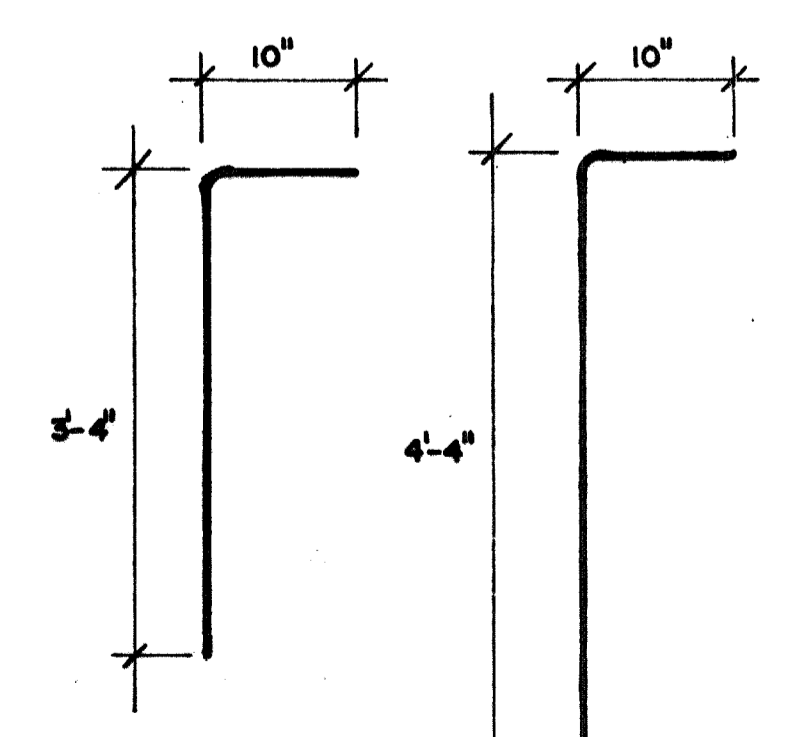


FOUNDATION

CONTINUITY REINFORCEMENT

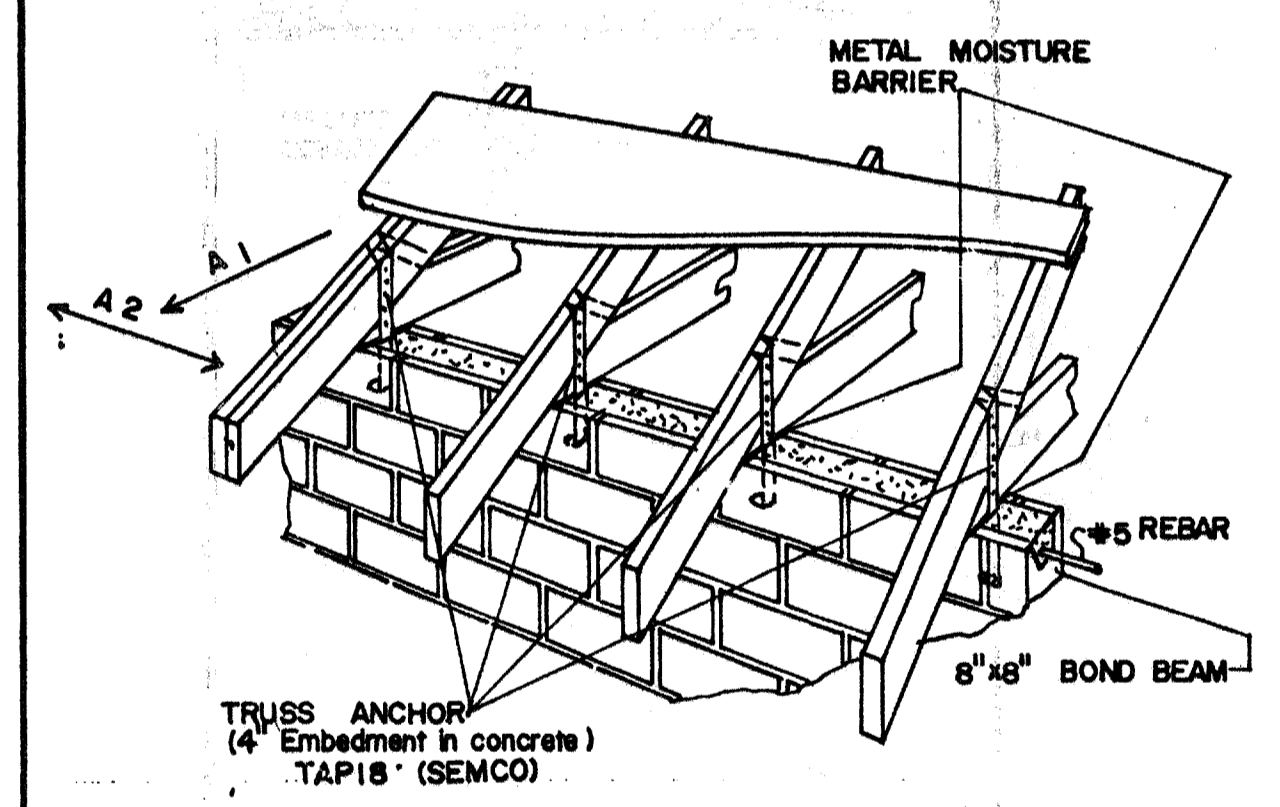


STEMWALL FOUNDATION OPTION



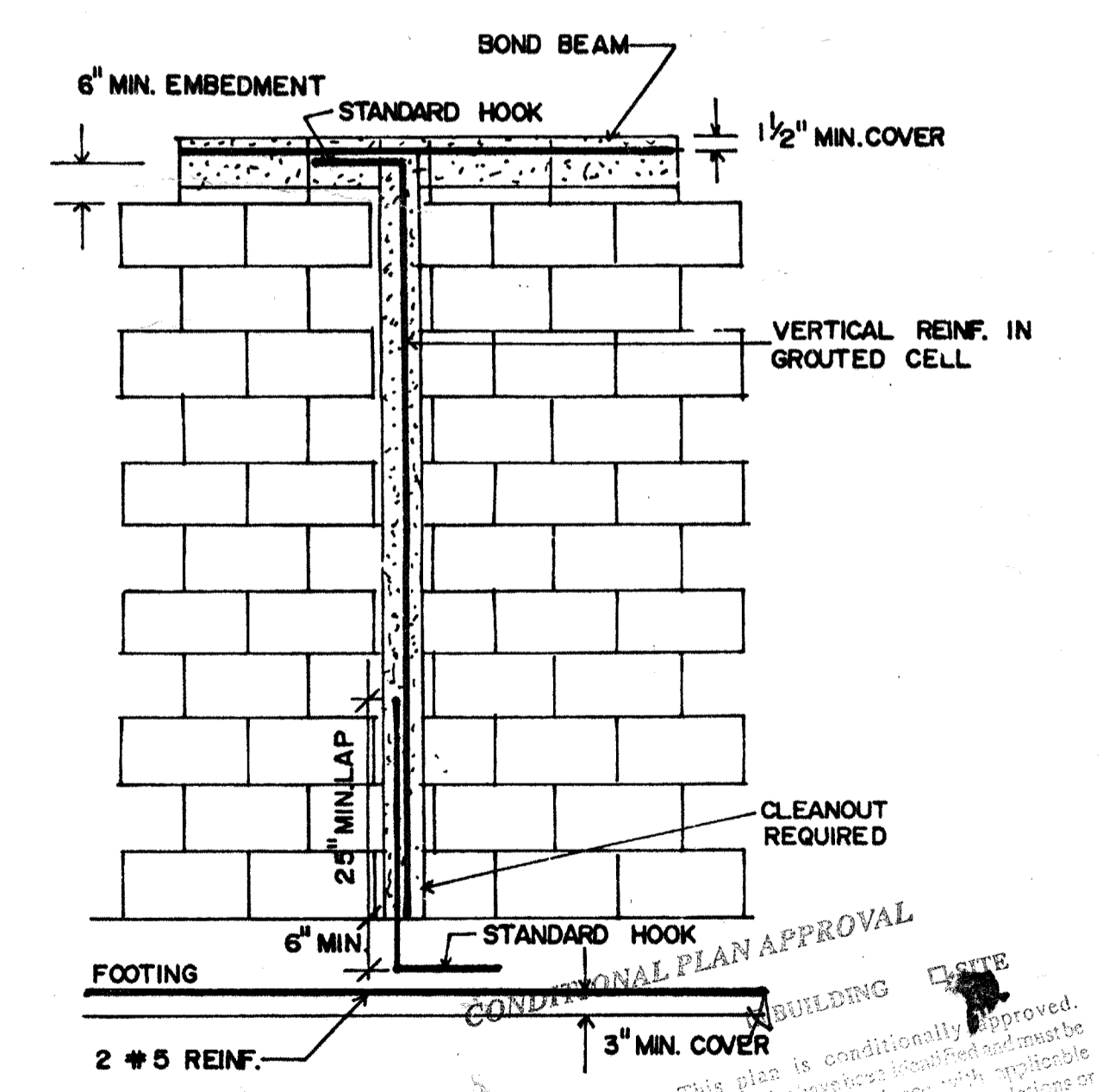
DOWEL FOR MONOLITHIC SLAB

DOWEL FOR STEMWALL FOUND.



LATERAL LOADS
A 1 = 595
A 2 = 210
UPLIF LOAD = 910 TO 1736 lbs

TRUSS CONNECTION DIRECT TO BOND BEAM



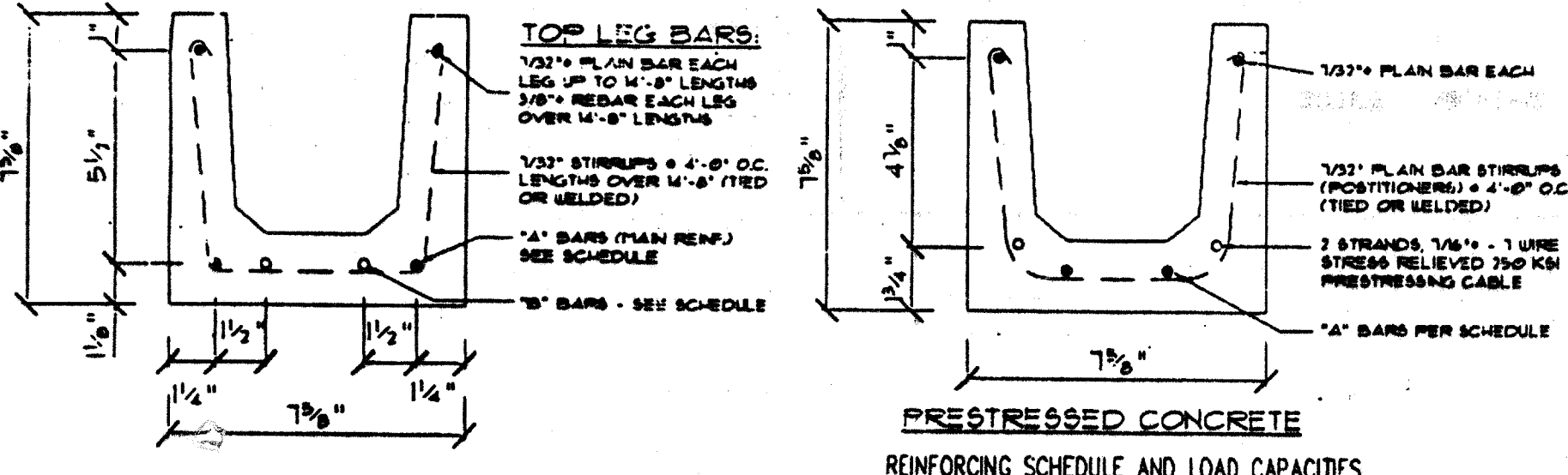
VERTICAL WALL CONT. REINF.

ARCHITECTURAL DESIGN-DRAFTING
CARLOS CASTILLA *Carroll*
813-266-8860 TAMPA FL

DATE: _____
JOB NO: _____
DESIGN BY: _____
DRAWN BY: _____
CHECKED BY: _____
Sheet **D1**

This plan is conditionally approved. It is not to be construed as a final approval. The contractor must provide a copy of this plan to the building department for their review and approval. This approval shall not be construed as a final approval. The contractor must provide a copy of this plan to the building department for their review and approval.

TYPICAL LINTEL SECTIONS



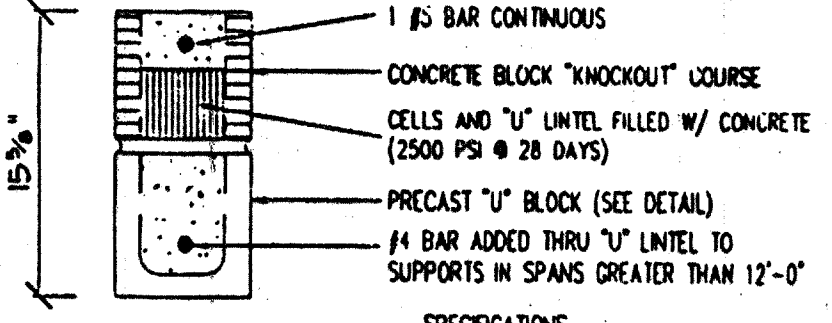
PRESTRESSED CONCRETE REINFORCING SCHEDULE AND LOAD CAPACITIES

LINTEL LENGTH	CLEAR SPAN	"A" BAR SCHEDULE	MAX SAFE LOAD LINTEL ONLY	SAFE LOAD ON COMPOSITE
14'-0"	13'-4"	NONE	653 /FT.	1,900 /FT.
15'-4"	14'-0"	NONE	650 /FT.	1,810 /FT.
17'-4"	16'-0"	#4 BARS	570 /FT.	1,580 /FT.
19'-4"	18'-0"	2 #5 BARS	500 /FT.	1,400 /FT.
20'-0"	18'-8"	2 #5 BARS	425 /FT.	1,400 /FT.
22'-0"	20'-8"	2 #5 BARS	380 /FT.	1,220 /FT.
24'-0"	22'-8"	2 #5 BARS	320 /FT.	1,120 /FT.

PRECAST CONCRETE REINFORCING SCHEDULE AND LOAD CAPACITIES

LINTEL LENGTH	BAR LENGTH	CLEAR SPAN	BOTTOM REINFORCING BARS "A" MAIN BARS	"B" BARS	MAX SAFE LOAD LINTEL ONLY	SAFE LOAD ON COMPOSITE
2'-0"	2'-3"	1'-6"	2 #7/32"	NONE	1,000+	3,000+
3'-0"	3'-4"	2'-0"	2 #7/32"	NONE	1,000+	3,000+
4'-0"	3'-7"	2'-6"	2 #7/32"	NONE	1,000+	3,000+
4'-6"	4'-4"	3'-0"	2 #7/32"	NONE	1,000+	3,000+
5'-4"	5'-2"	4'-0"	2 #5	NONE	1,000+	2,100+
5'-10"	5'-8"	4'-6"	2 #5	NONE	1,000+	2,100+
6'-4"	6'-2"	5'-0"	2 #5	NONE	1,000+	2,100+
6'-10"	6'-8"	5'-6"	2 #5	NONE	1,000+	2,100+
7'-6"	7'-0"	6'-2"	2 #5	NONE	800+	2,250+
8'-4"	7'-8"	7'-0"	2 #5	NONE	800+	2,250+
10'-6"	10'-0"	9'-2"	2 #5	NONE	800+	2,250+
11'-4"	10'-10"	10'-0"	2 #5	2 #5	800 /FT.	2,000 /FT.
12'-6"	12'-0"	11'-2"	2 #5	2 #5	700 /FT.	1,800 /FT.
13'-4"	12'-10"	12'-0"	2 #5	2 #5	650 /FT.	1,600 /FT.
14'-0"	13'-6"	12'-8"	2 #5	2 #5	600 /FT.	1,700 /FT.

COMPOSITE LINTEL SECTION

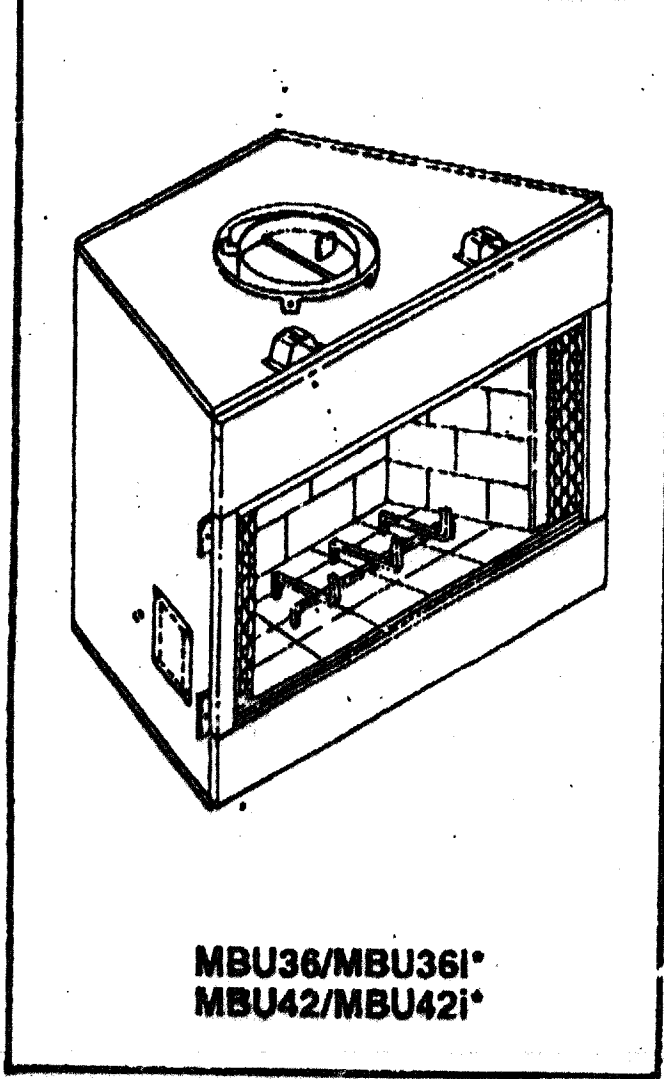
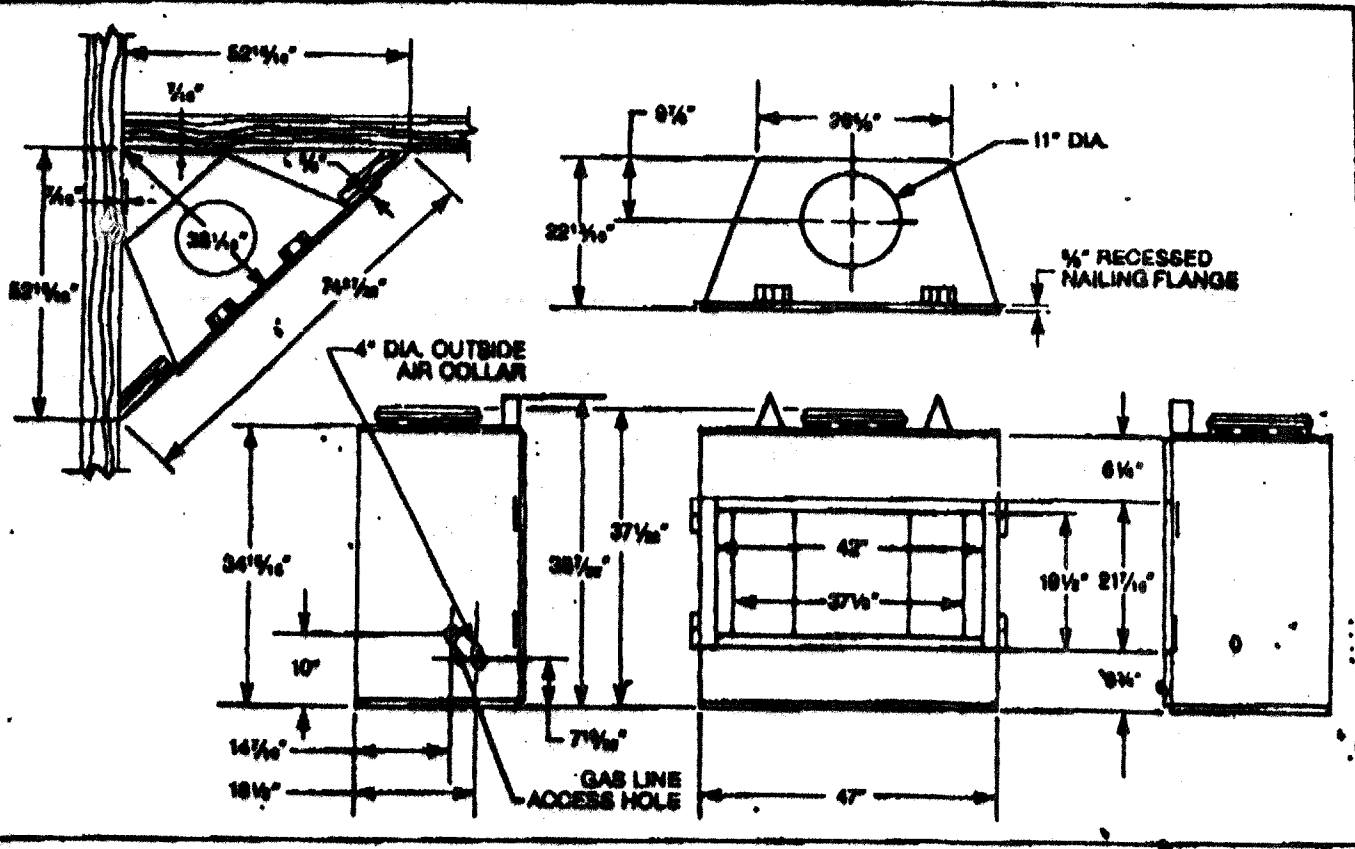


1. BAR STEEL SHALL BE MIN. GRADE 40, DEFORMED EXCEPT WHERE PLAN BARS (HOOKED FOR BOND ANCHORAGE) ARE SPECIFICALLY PERMITTED.
 2. CONCRETE IN PRECAST MEMBERS TO BE MIN. 4000 PSI @ 28 DAYS.

Safe Load Table For Cast-Crete High-Strength Precast & Prestressed U Lintels

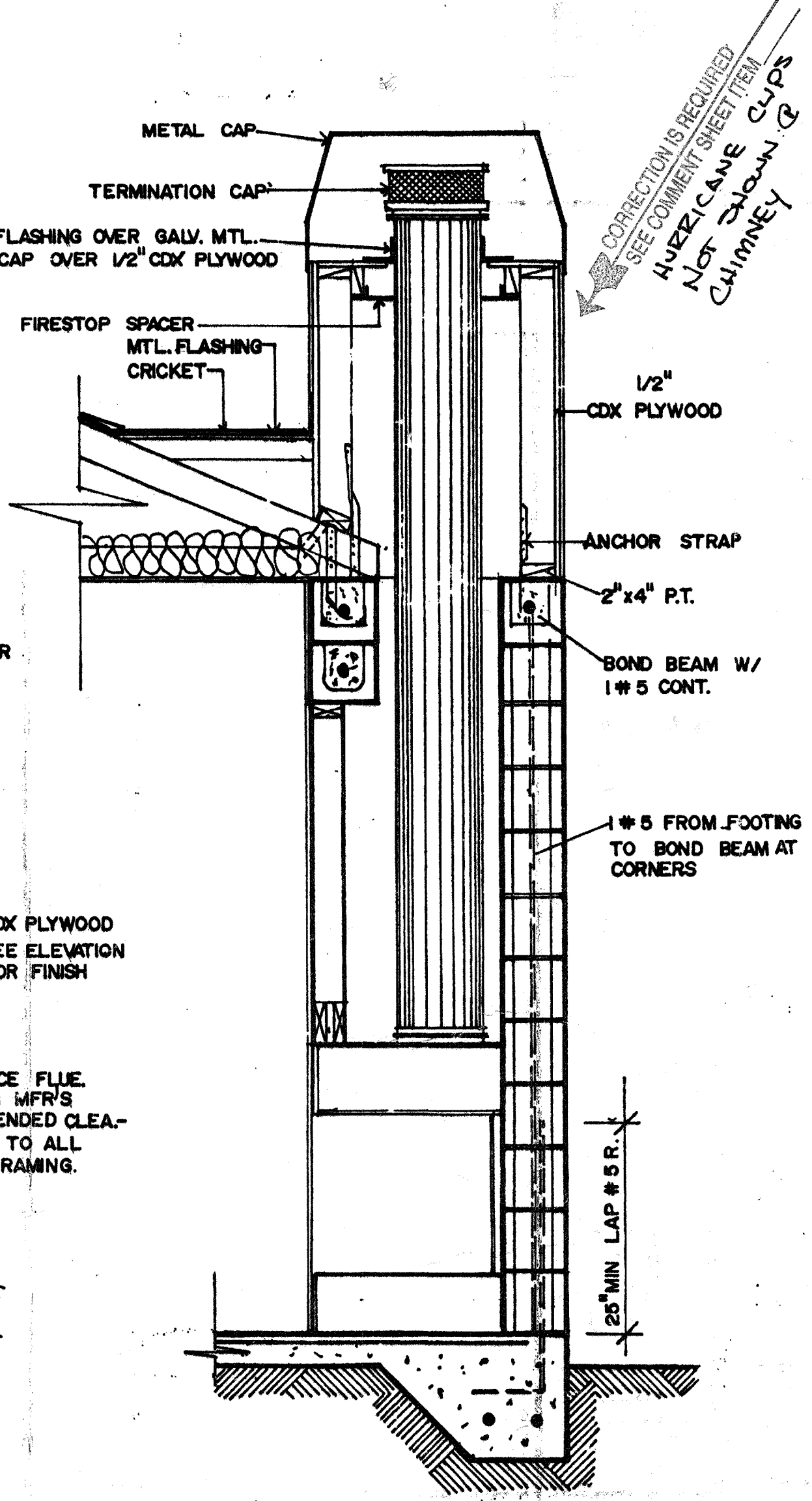
No.	LENGTH	SAFE LOAD - POUNDS PER LINEAR FOOT	
		UNFILLED	FILLED
L 1	4'-0" (48") Precast	1,815 Lbs.	6,048 Lbs.
L 2	4'-6" (54") Precast	1,850 Lbs.	4,815 Lbs.
L 3	5'-0" (60") Precast	1,743 Lbs.	4,615 Lbs.
L 4	5'-6" (66") Precast	1,575 Lbs.	3,744 Lbs.
L 5	6'-0" (72") Precast	1,565 Lbs.	3,260 Lbs.
L 6	7'-0" (84") Precast	1,550 Lbs.	2,847 Lbs.
L 7	8'-0" (96") Precast	1,025 Lbs.	1,747 Lbs.
L 8	10'-0" (120") Precast	922 Lbs.	1,333 Lbs.
L 9	11'-0" (132") Precast	800 Lbs.	1,483 Lbs.
L 10	12'-0" (144") Precast	750 Lbs.	1,304 Lbs.
L 11	13'-0" (156") Precast	851 Lbs.	1,018 Lbs.
L 12	14'-0" (168") Precast	585 Lbs.	1,341 Lbs.
L 13	14'-0" (176") Prestressed	550 Lbs.	1,241 Lbs.
L 14	15'-0" (184") Prestressed	510 Lbs.	1,210 Lbs.
L 15	17'-0" (204") Prestressed	387 Lbs.	911 Lbs.
L 16	19'-0" (228") Prestressed	289 Lbs.	820 Lbs.

Above information is based on Cast-Crete (FCCP Corp.) brand lintels. Based on tests conducted by Werner F. Rosch, P.E.
 Precast lintels may be supplied by Cast-Crete or other building code approved precast manufacturers.



FIREPLACE DETAILS

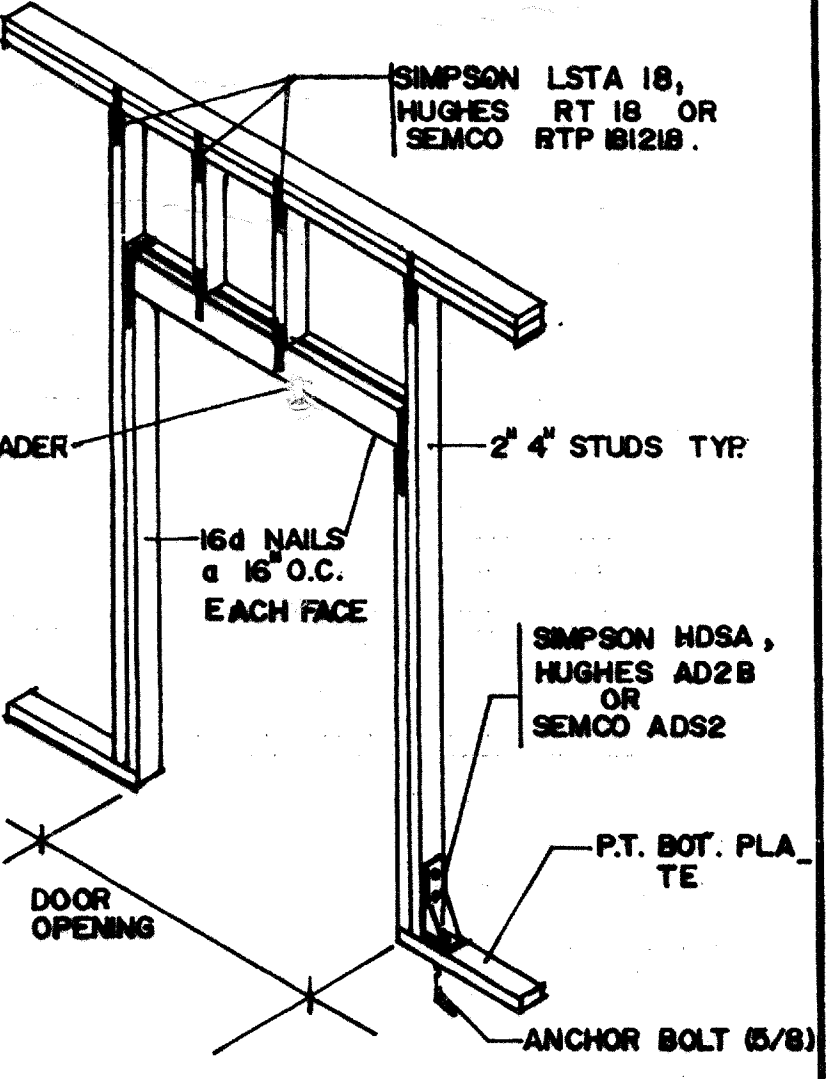
SECTION @ CHIMNEY



- GLASS BLOCK INSTALLATION ***
- A) COVER SILL AREA WITH A HEAVY COAT OF ASPHALT EMULSION. ALLOW EMULSION TO DRY AT LEAST 2 HOURS BEFORE PLACING MORTAR.
 - B) ADHERE EXPANSION STRIPS TO JAMBS AND HEAD. MAKE CERTAIN EXPANSION STRIP EXTENDS TO SILL.
 - C) SET A FULL MORTAR BED JOINT, APPLIED TO SILL.
 - D) SET LOWER COURSE OF BLOCK. MAINTAIN A UNIFORM JOINT WIDTH OF 1/4" PLUS OR MINUS 1/8". ALL MORTAR JOINTS MUST BE FULL AND NOT FURROWED. STEEL TOOLS MUST NOT BE USED TO TAP BLOCKS INTO POSITION. (PLACE 1/2" RUBBER CRITCH TIP ON END OF TROWEL TO TAP BLOCK INTO POSITION.) DO NOT REALIGN, TAP, OR OTHERWISE MOVE BLOCK AFTER INITIAL PLACEMENT. FOR SOLID GLASS BLOCK IT MAY BE NECESSARY TO USE WEDGES IN THE MORTAR JOINTS OF THE LOWER COURSES TO PREVENT THE MORTAR FROM BEING "SQUEEZED" OUT.
 - E) INSTALL PANEL REINFORCING ON 24 INCH CENTERS HORIZONTALLY - 16 INCH CENTERS FOR THINLINE SERIES GLASS BLOCK AND SOLID GLASS BLOCK UNITS - AND IN JOINTS IMMEDIATELY ABOVE AND BELOW ALL OPENINGS WITHIN PANELS. (THE SAME SPACING SHALL APPLY TO PANEL ANCHORS WHEN USED AT JAMBS AND HEADS IN LIEU OF CHANNEL OR CHASE SURROUNDS.) RUN REINFORCING CONTINUOUSLY FROM END TO END OF PANELS. LAP REINFORCING NOT LESS THAN 6 INCHES WHENEVER IT IS NECESSARY TO USE MORE THAN ONE LENGTH. DO NOT BRIDGE EXPANSION JOINTS WITH REINFORCING. INSTALL REINFORCING AS FOLLOWS:
 - PLACE LOWER HALF OF MORTAR IN BED JOINT. DO NOT FURROW.
 - PRESS PANEL REINFORCING INTO PLACE.
 - COVER PANEL REINFORCING WITH UPPER HALF OF MORTAR BED AND TROWEL SMOOTH. DO NOT FURROW.
 - F) PLACE FULL MORTAR BED FOR JOINTS NOT REQUIRING PANEL REINFORCING - DO NOT FURROW. MAINTAIN UNIFORM JOINT WIDTH.
 - G) SET SUCCEEDING COURSE OF BLOCKS. SPACE AT HEAD OF PANEL AND JAMBS MUST REMAIN FREE OF MORTAR.
 - H) STRIKE JOINTS SMOOTH WHILE MORTAR IS STILL PLASTIC AND BEFORE FINAL SET. AT THIS TIME RAKE OUT ALL SPACES REQUIRING SEALANT TO A DEPTH EQUAL TO THE WIDTH OF THE SPACES. REMOVE SURPLUS MORTAR FROM FACES OF GLASS BLOCKS AND WIPES DRY. TOOL JOINTS SMOOTH AND CONCAVE, BEFORE MORTAR TAKES FINAL SET. (REMOVE WEDGES FROM LOWER COURSES OF SOLID BLOCKS AND POINT THE VOIDS WITH MORTAR.)
 - I) AFTER FINAL MORTAR SET (APPROX. 24 HOURS), INSTALL PACKING TIGHTLY BETWEEN GLASS BLOCK PANEL AND JAMB AND HEAD CONSTRUCTION. LEAVE SPACE FOR SEALING.
 - J) APPLY SEALANT EVENLY TO THE FULL DEPTH OF RECESSES IN ACCORDANCE WITH THE MANUFACTURER'S APPLICATION MANUAL AND INSTRUCTIONS.
 - K) A MAXIMUM OF 80 SQUARE FEET OF GLASS BLOCK MAY BE INSTALLED IN ANY OPENING. FOR WIND SPEEDS UP TO 110 MPH, PER PITTSBURGH CORNING RECOMMENDATIONS FOR INSTALLATION OF GLASS BLOCK.
- * SPECIFICATIONS FROM PITTSBURGH CORNING CORPORATION PUBLICATION: CB-165 30M REV. 11/88

GENERAL NOTES

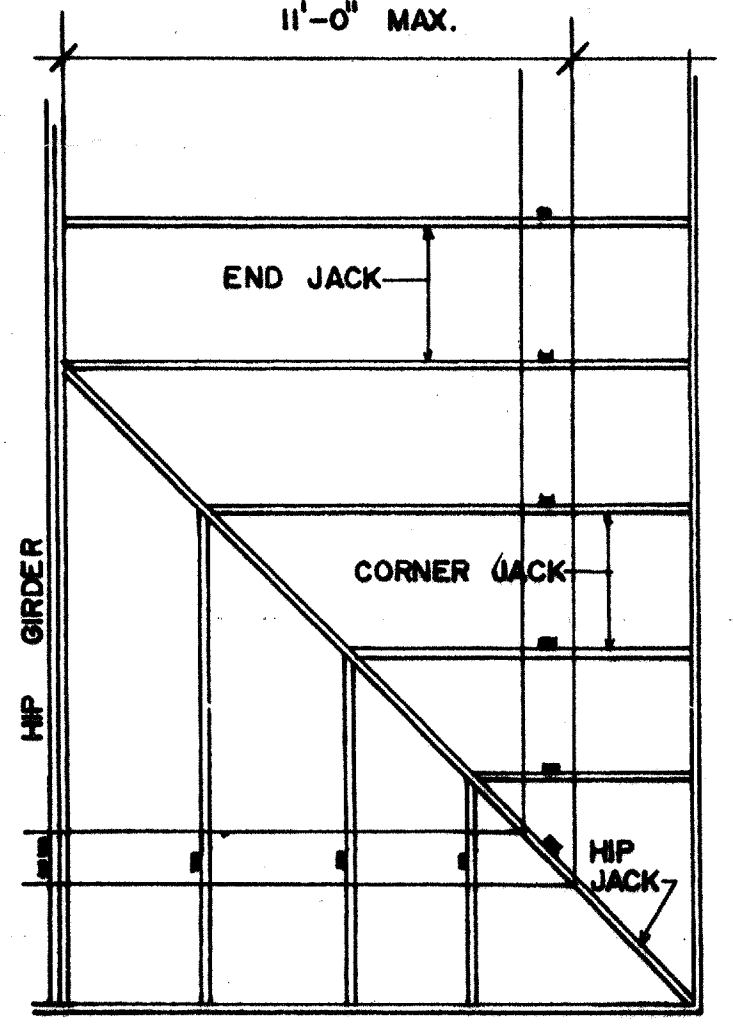
- Concrete shall have a minimum compressive strength of 2,500 psi at 28 days for footings, slabs and 3,000 psi at 28 days for tie beams.
- Where shown on drawings, core of block masonry shall be filled with grout or pea gravel concrete with a minimum compressive strength of 3,000 psi at 28 days.
- Reinforcing steel shall be ASTM A-615 GR60. Clear cover shall be 3" for footings and 1 1/2" for tie beams.
- Concrete masonry units shall be ASTM C90-78, hollow core bearing concrete masonry units. Type 1, grade N-1, normal weight.
- Mortar shall conform to ASTM C270 and be type M or S.
- Dowels shall have a standard 90 degree hook, 10" long and shall embed 6" minimum into the footing and lap 25" minimum with vertical rebar. Vertical rebar shall have a standard 90 degree hook and embed in the tie beam top face.
- Design loads are in accordance with SBCCI 1991 code and local codes as they may apply: Wind Load 100 mph at 30' above grade. Floor Live Load 40 psf. Roof Live Load 30 psf. Roof Dead Load 10 psf. Soil Bearing Pressure minimum 2,000 psf.
- Fill material shall be compacted to 95% and tests are required for all fill of 24" or more measured from the existing grade to the top of fill. All structural foundations shall be carried to existing grade.
- All structural wood framing members shall be #2 Southern Yellow Pine, or better, and shall have a minimum extreme fiber bending stress of at least 1,150 psi. All interior studs shall be Spruce or better and have a minimum extreme fiber bending stress of 1,350 psi. All wood in contact with concrete and/or exposed to the weather shall be pressure-treated lumber.
- Steel plates and shapes shall conform to ASTM A36. Bolts, nuts and washers shall conform to ASTM A307. All material shall be hot dipped galvanized steel.
- Manufactured products shall be installed according to manufacturer's recommendations and erection drawings.
- Contractor shall field verify all dimensions.



UPLIFT CONNECTIONS AT LOAD BEARING WALL

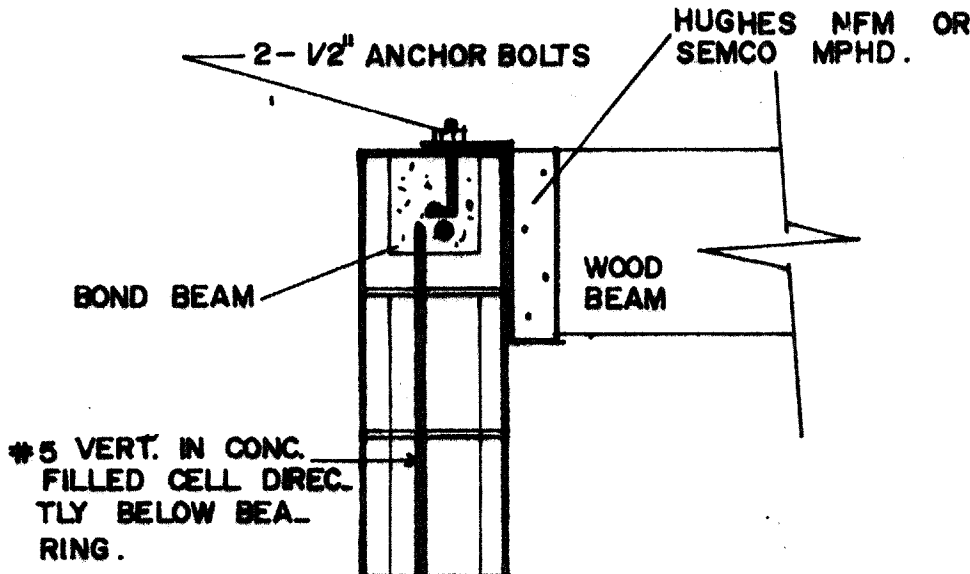
INT. HEADER SCHEDULE	HEADER
UP TO 3'-6"	2-2x16"
3'-6" TO 6'-4"	2-2x18"
6'-4" TO 8'-6"	2-2x20"
8'-6" TO 12'-6"	2-2x12"

USE SIMPSON ETA 14 OR HUGHES TA 20 OR SEMCO TAP 14



UPLIFT CONNECTIONS FOR TYPICAL HIP FRAMING

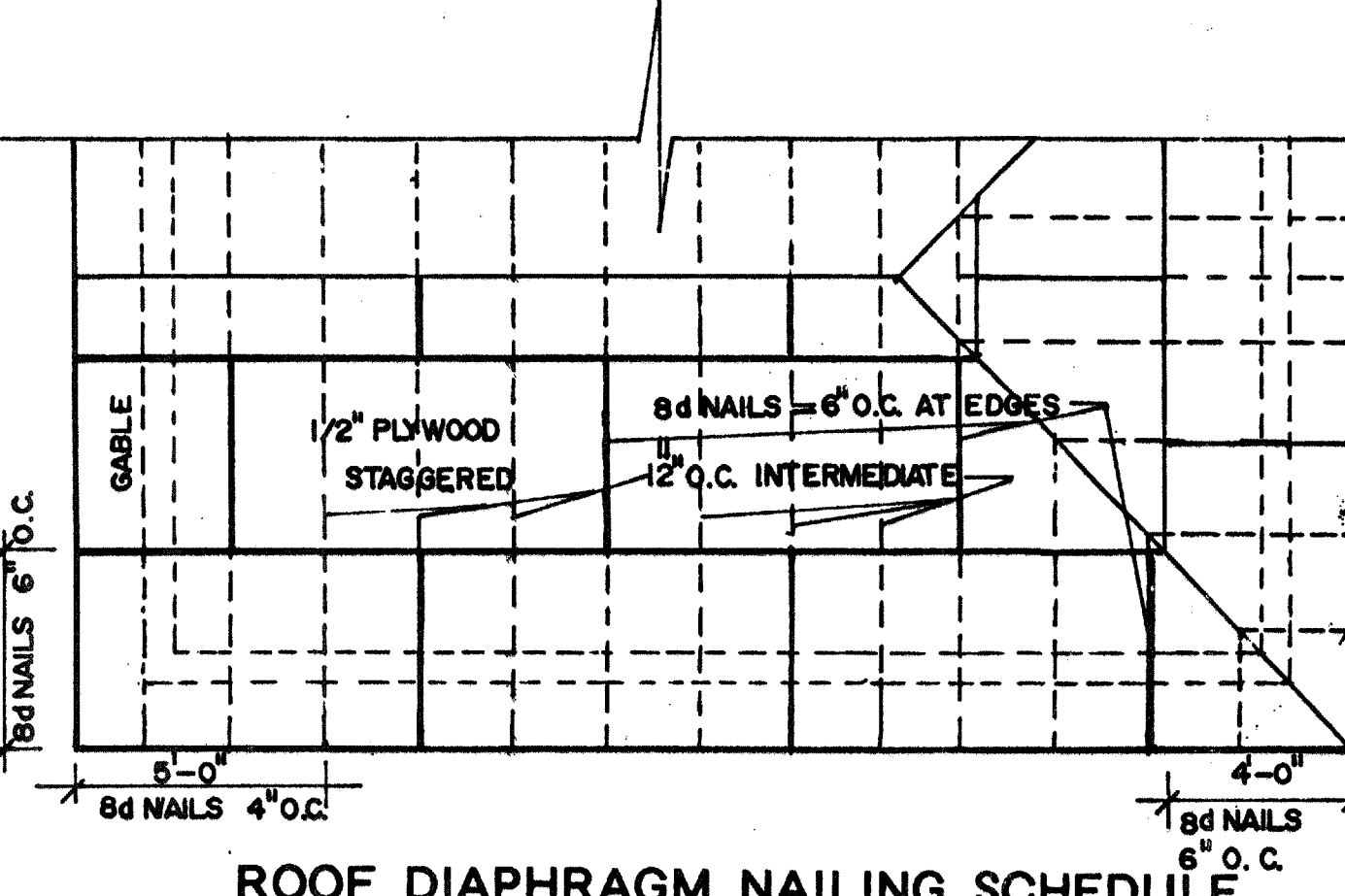
SCALE 3/8"=1'-0"



TYPICAL BEAM TO C.M.U. WALL CONNECTION

NAILING SCHEDULE

Ledger Strip	16d common	3 at each joist
Sole Plate to joist or blocking, face nail	16d common	16" OC
Top or Sole Plate to stud end nailed	16d common	2
Stud to sole plate, toe nailed	3d common	4
Double studs, face nailed	10d common	24" OC
Doubled top plate face nail	10d common	16" OC
Continuous header, toe pieces	16d common	16" OC along each edge
Continuous header, toe nail	8d common	3
1x8 sheathing or less to each bearing, face nail	8d common	2
Over 1x8 sheathing to each bearing, face nail	8d common	3
Build-up corner studs	16d common	24" OC
Build-up girders and beams up to three members	20d common	32" OC at top and bottom and staggered 2 ends and at each splice
1/2" Gypsum sheathing	11 ga 1 1/2" galv	4" OC at edges
5/8" Gypsum sheathing	7/16" head	8" OC at other bearings
Gypsum wallboard 1/2"	1 3/8" drywall nail	7" OC on ceiling
		8" OC on walls
		7" OC on ceiling
		8" OC on walls
		8" OC on walls
		8" OC on walls
		8" OC on walls
		8" OC on walls



ROOF DIAPHRAGM NAILING SCHEDULE

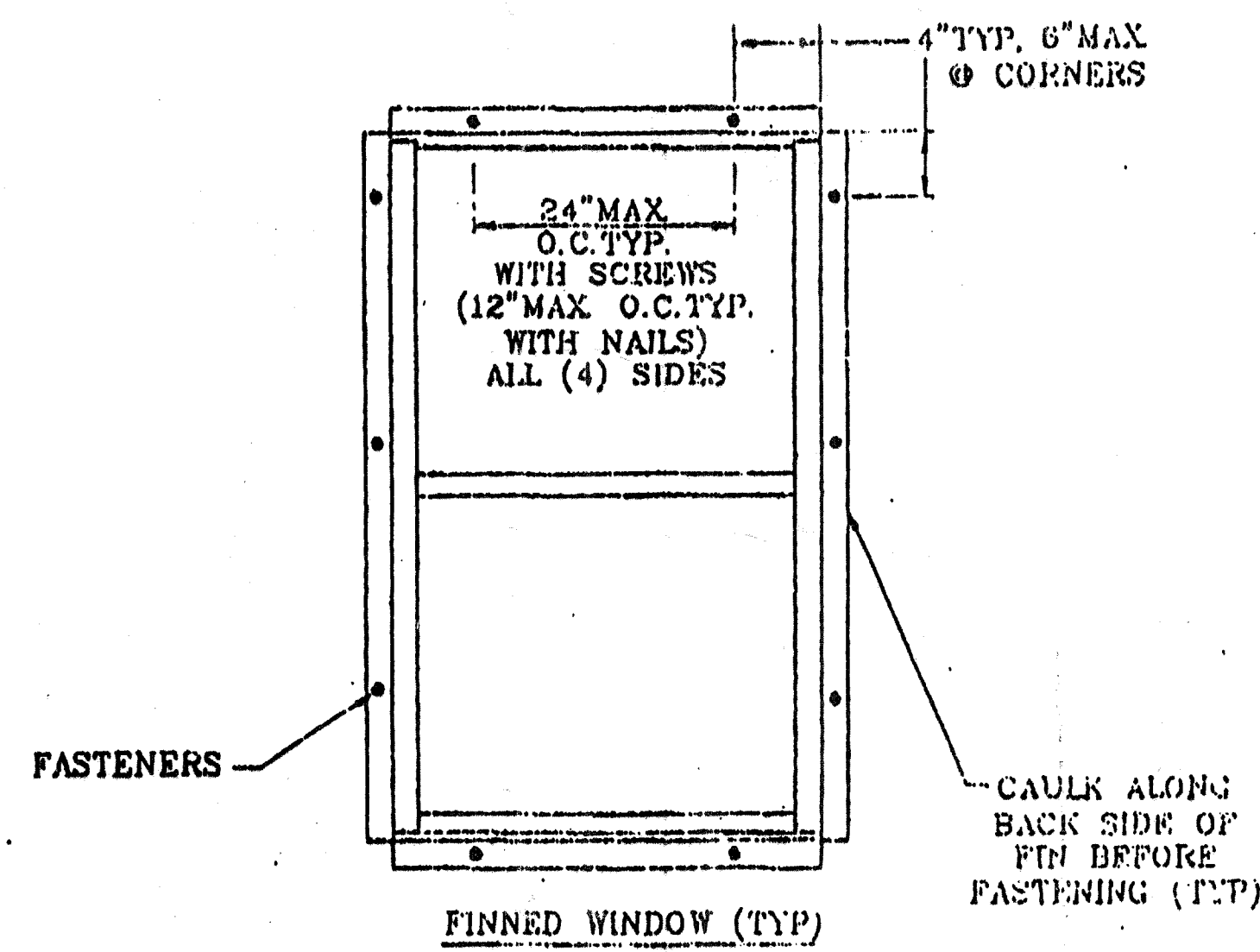
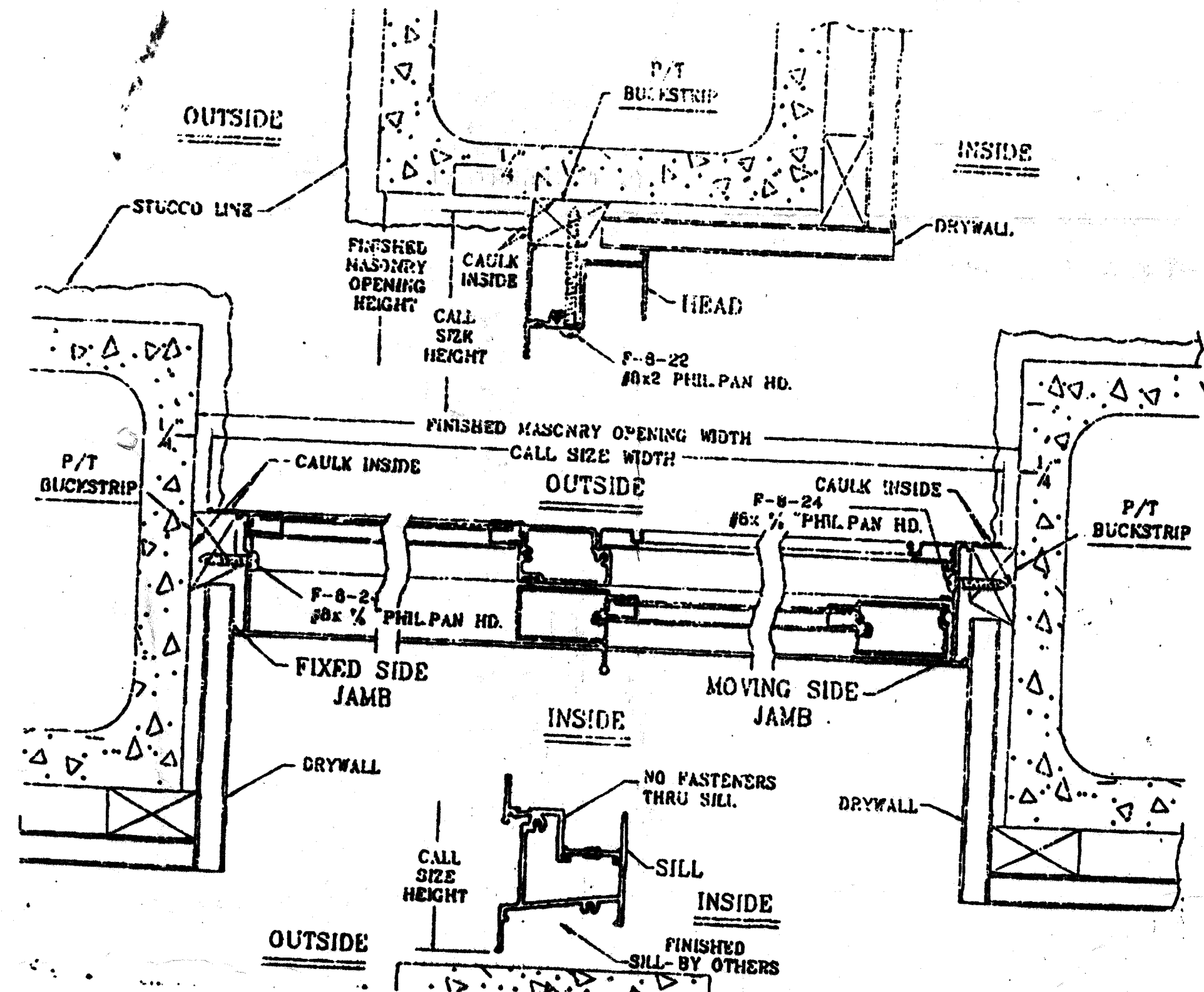
ARCHITECTURAL DESIGN-DRAFTING

CARLOS CASTILLA

613-265-8880 TAMPA FL

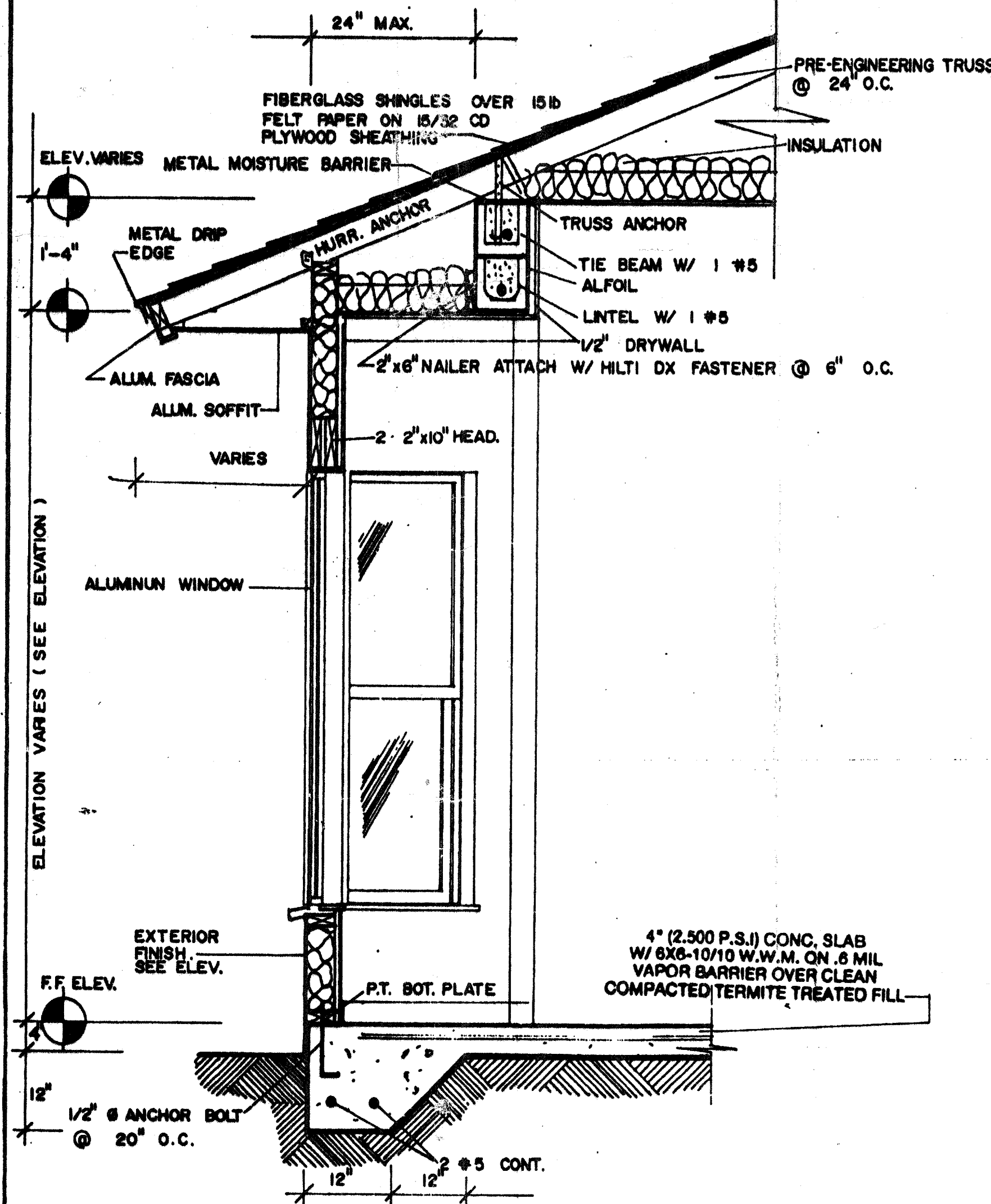
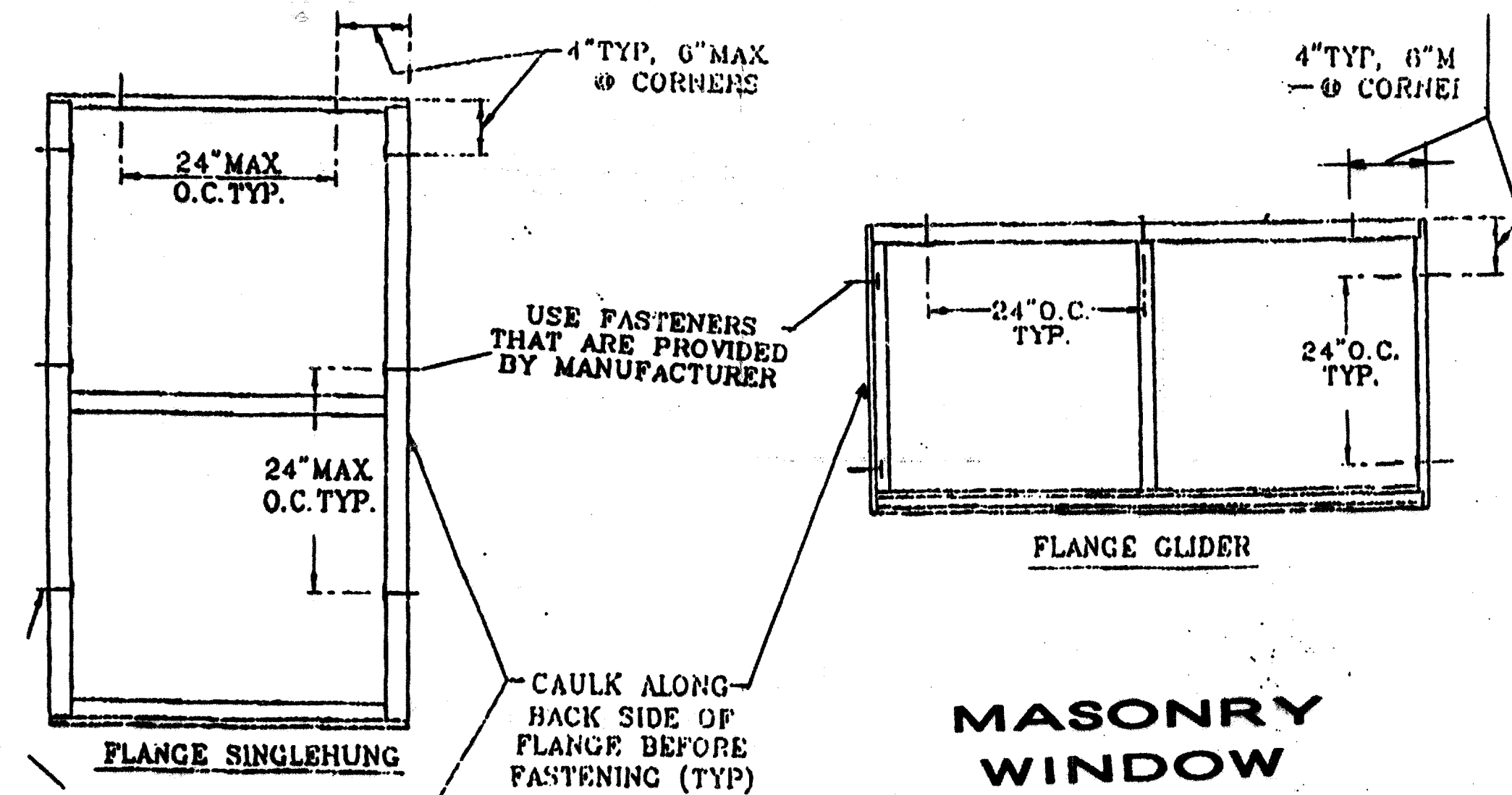
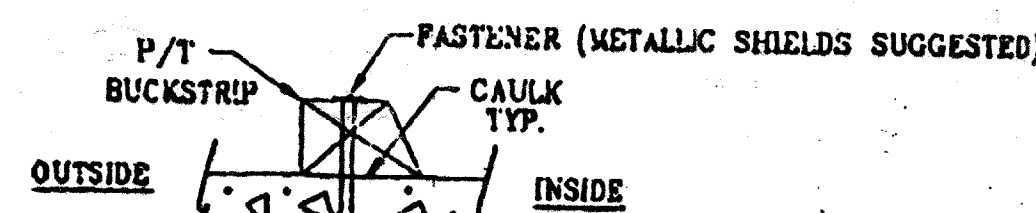
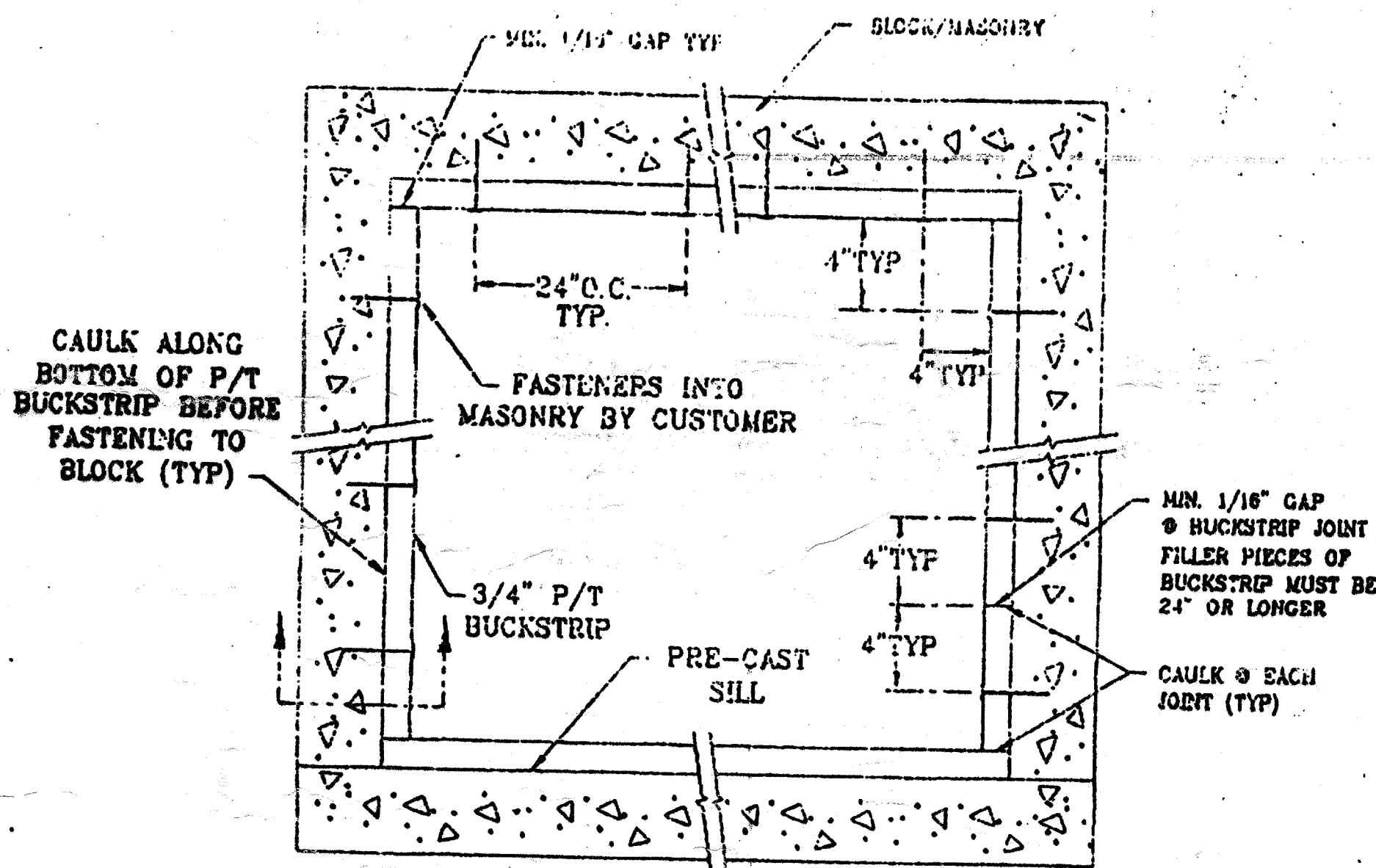
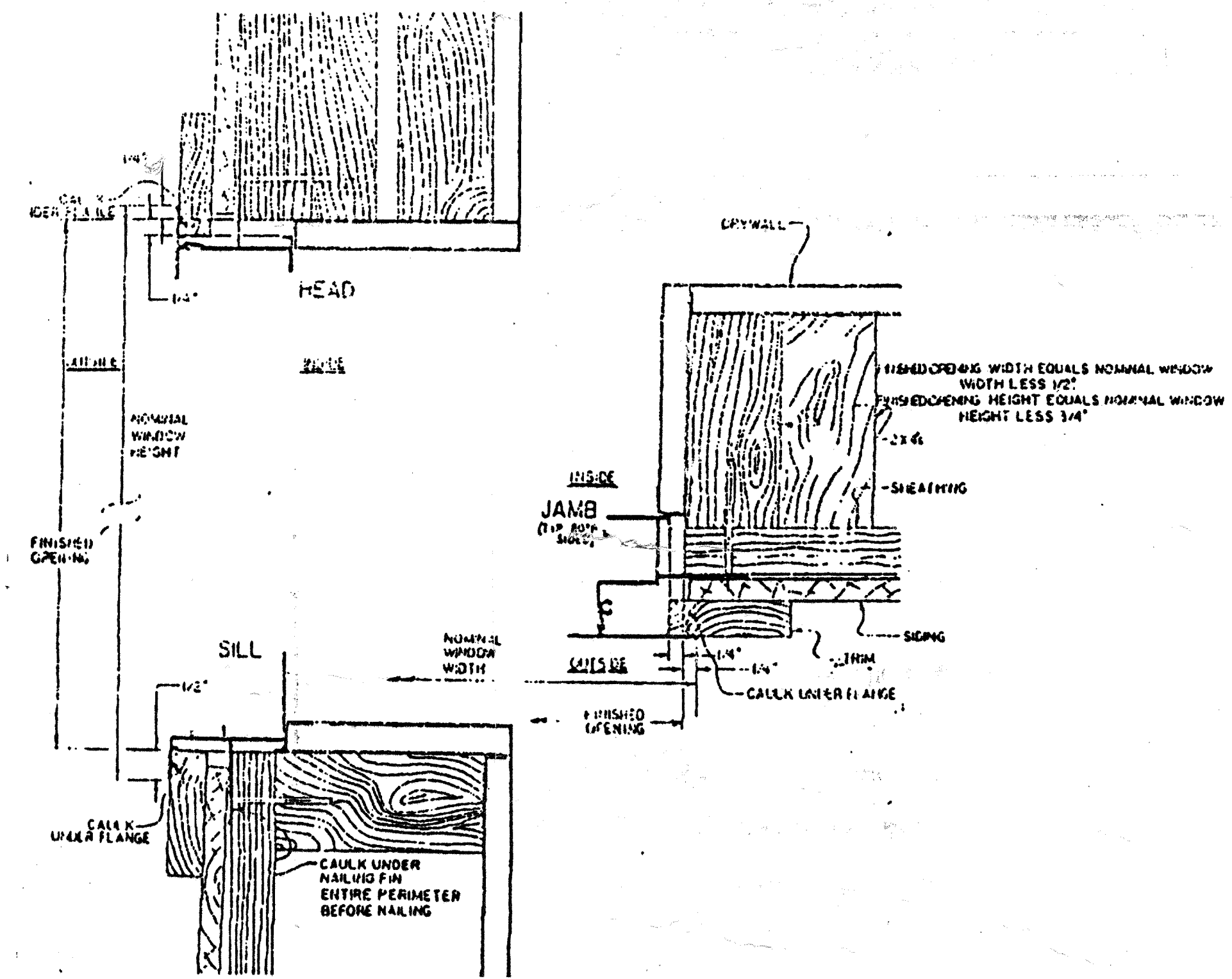
ADDITIONAL PLAN APPROVAL
 BUILDING SITE
 This plan is conditionally approved. It is subject to the approval of the City of Tampa. The contractor shall be responsible for obtaining all necessary permits and approvals from the appropriate authorities. The contractor shall be responsible for maintaining the status of this conditional approval. The contractor shall not be construed as a guarantee of any kind, either in whole or in part. The contractor shall be responsible for obtaining all necessary permits and approvals from the appropriate authorities. The contractor shall be responsible for maintaining the status of this conditional approval.

DATE: _____
 JOB NO: _____
 DESIGN BY: _____
 DRAWN BY: _____
 CHECKED BY: _____
 Sheet **D2**



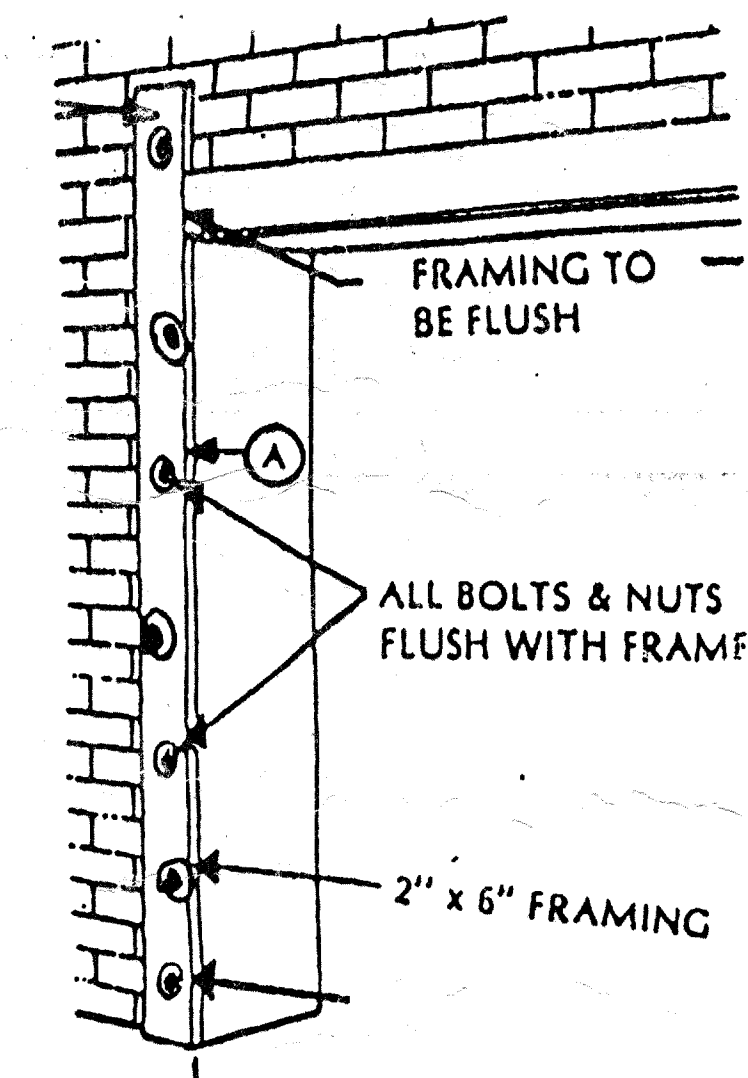
- NOTES:**
1. USE #10 x 1" PH. SMS, #10 x 1" WOOD SCREWS OR 8d GALVANIZED NAILS.
 2. ATTACH WINDOW DIRECTLY TO FRAMING UNDER SUB-SIDING.

WOOD FRAME WINDOW



SECTION THROUGH BAY WINDOW

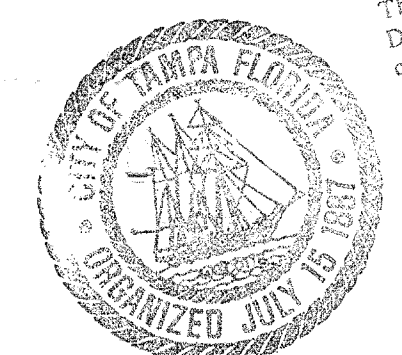
SCALE 3/4"=1'-0"



GARAGE DOOR ATTACHMENT

(100 MPH)

CONDITIONAL PLAN APPROVAL



This plan is conditionally approved. Deficiencies have been identified and must be corrected for compliance with applicable federal, state, county and city regulations or codes. These deficiencies must be corrected prior to construction commencement. The City of Tampa assumes no liability whatsoever in the issuance of this conditional approval. This approval shall not be construed as authority to violate, cancel, alter or set aside provisions of any federal, state, county or city regulation or code.

ARCHITECTURAL DESIGN-DRAFTING

CARLOS CASTILLA

813-266-8800 TAMPA FL

DATE: _____
 JOB NO: _____
 DESIGN BY: _____
 DRAWN BY: _____
 CHECKED BY: _____

Sheet **D3**